

Article

Microsurfacing Pavement Solutions with Alternative Aggregates and Binders: A Full Surface Texture Characterization

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Abstract: The road surface texture is responsible for controlling several quality/safety road indicators, such as friction, noise, and fuel consumption. Road texture can be classified into different wavelengths, and it is dependent on the material used in the paving solution. With the aim of evaluating and characterizing the surface texture of a microsurfacing road pavement, six microsurfacing samples were made in the laboratory with both traditional materials (basaltic aggregates and bituminous emulsion) and with innovative materials from recycling procedures (crumb rubber (CR) and artificial engineered aggregate (AEA)). The characterization was performed through the use of a conoscopic holography profilometer with high precision and post-processing of the profiles detected through consolidated algorithms (ISO standards). We found that the aggregate type plays a very important role in the pavement texture. The binder agent seems to be highly important, but more studies regarding this are necessary. The use of crumb rubber as an aggregate proved to be feasible, and the texture parameters that were obtained were in accordance with the benchmark ones. In addition, the study shows that the use of artificial engineered aggregates does not impair the surface texture. Finally, the use of the texture parameters defined by the ISO standards, together with a statistical analysis, could be useful for defining the surface texture characteristics of microsurfacing.

Keywords: microsurfacing; surface texture; MPD; crumb rubber; artificial aggregate; geopolymer



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1. Introduction

The texture of the road pavement is directly responsible for several indicators, namely: internal and external noise [1–3], safety [4,5], heat island mitigation [6–8], fuel consumption [4], tire wear, and overall costs of road operation. All of these aspects are regulated in one or another fashion by the texture of the pavement.

The pavement can be classified according to the texture wavelength [9], which starts from the unevenness of the road and megatexture, through the surface macrotexture up to the microtexture of the aggregate itself. Macrotexture and microtexture scales mostly impact the tire/road interaction and are related to many functional characteristics of the pavement, such as the drainability, friction, and noise, which affect the safety, social, and environmental costs [10–12].

The pavement texture is a product of the mix of a binder (bitumen, resin, Portland cement, and asphalt emulsion), fillers, and aggregates. The mix design and aggregate origin play a major role in the pavement texture [11,13–17], and consequently in noise and friction [18]. The surface texture of the pavement can be measured using different techniques, both in the laboratory and in the field. The main methods are volumetric, laser, and permeability [19–22]. Photogrammetric techniques have been implemented and are showing promising results [19,23–25]. Texture can also be evaluated using tribological parameters, such as skewness, kurtosis, and the Abbott curve [26–29].

One of the possible solutions to improve the surface texture of a pavement is microsurfacing, sometimes called slurry sealing [30]. Microsurfacing is a preventive maintenance technology that involves the application of a mixture consisting of polymer-modified bituminous emulsion, dense-graded mineral aggregates, mineral filler, water, and additives (if needed) at an ambient temperature. Several studies have been conducted to assess the properties of microsurfacing regarding noise and friction [31–34]. Recently, more and more research has focused on substituting either the asphalt binder [17,35,36] or replacing the natural aggregate with alternative aggregates, such as steel slag, recycled materials, rubber, and alkali-activated or geopolymeric materials [17,37–40].

The present study aimed to carry out a detailed texture analysis of six different microsurfacing mixtures by means of a laser profilometer. The experiment was designed to assess the use of two different binders (asphalt emulsion and resin) and three different aggregates (natural aggregate, 3D printed artificial geopolymeric aggregate, and crumb rubber) by evaluating the pavement surface texture features of the mixtures. The texture analysis was carried out not only through the use of traditional parameters, but also using statistical/tribological parameters.

2. Materials and Methods

2.1. Materials

Three types of aggregates were used in this study:

- Natural aggregate (NA)—Basaltic origin—0/6 or 0/8 mm (the latter is obtained from a combination of 0/6 and 4/8, whose physical and mechanical properties are described in Table 1);
- Crumb rubber (CR)—0.8/4 mm [41,42];
- Artificial engineered aggregate (AEA)—10 mm single size [40].

Table 1. Physical and mechanical properties of the natural aggregate.

Properties	Specification	Basaltic Aggregate 0/6	Basaltic Aggregate 4/8
Bulk Specific Gravity [Mg/m ³]	UNI EN 1097-6	2.68	2.66
Crushed Particles [%]	UNI EN 933-5	100	100
Methylene Blue Test	UNI EN 933-9	0.8	-
Shape Index [-]	UNI EN 933-4	-	15
Flakiness Index [-]	UNI EN 933-3	-	10
Micro Deval [-]	UNI EN 1097-1	-	10
Los Angeles [%]	UNI EN 1097-2	-	20
Polished Stone Value [-]	UNI EN 1097-8	-	50

In particular, the artificial engineered aggregates were produced using an alkali-activation technique. It consists of mixing the basalt powder and metakaolin with an activator (alkali solution). This mixture produces a mortar that is poured into 3D printed molds and is cured in the oven. The final aggregate has a 10 mm diameter in a truncated octahedron shape, as shown in Figure 1.

For the scope of this research, six different microsurfacing mixtures were prepared, as summarized in Table 2. In order to make a comparison between the different mixture combinations, two benchmark samples were made, called the natural aggregate sample, “NAS1” and “NAS2”, produced with a 0/8 mm basaltic aggregate and modified asphalt emulsion, which represent a standard microsurfacing mixture.



Figure 1. Artificially engineered aggregate.

Table 2. Description of the analyzed samples.

Name	Type of Binder	Type of Aggregate	Granulometric Information
AA5050	Asphalt emulsion modified with latex	NA AEA	50% < sieve #2 of 0/6 NA 50% 10 mm single size AEA
AA	Asphalt emulsion modified with latex	NA AEA	Australian gradation curve (0/10 mm) replacing 10 mm NA by 10 mm AEA
Rubber	Asphalt emulsion modified with latex	NA CR	40% NA 60% 0/4 mm CR
NAS2	Asphalt emulsion modified with latex	NA	0/8 mm
Yellow	Non-asphaltic resin	NA	0/6 mm
NAS1	Asphalt emulsion modified with latex	NA	0/8 mm

Basalt aggregate with a 0/6 mm granulometric curve was used in the “Yellow” sample, replacing the modified asphalt emulsion with a commercial non-asphaltic resin binder, whose color is the name of the sample. The “Rubber” sample used asphalt emulsion, whereas the granulometric curve was built up using 40% NA and 60% CR [42]. The last two samples used a combination of both NA and AEA blended with asphalt emulsion. Sample “AA” used an Australian gradation curve (0/10 mm) [43], replacing the maximum aggregate size (retained at sieve 9.5 mm) with the AEA. Mixture “AA5050” is a discontinuous mix made with 50% of 0/6 mm NA (<sieve #2) and 50% AEA (10 mm). The samples are shown in Figure 2.

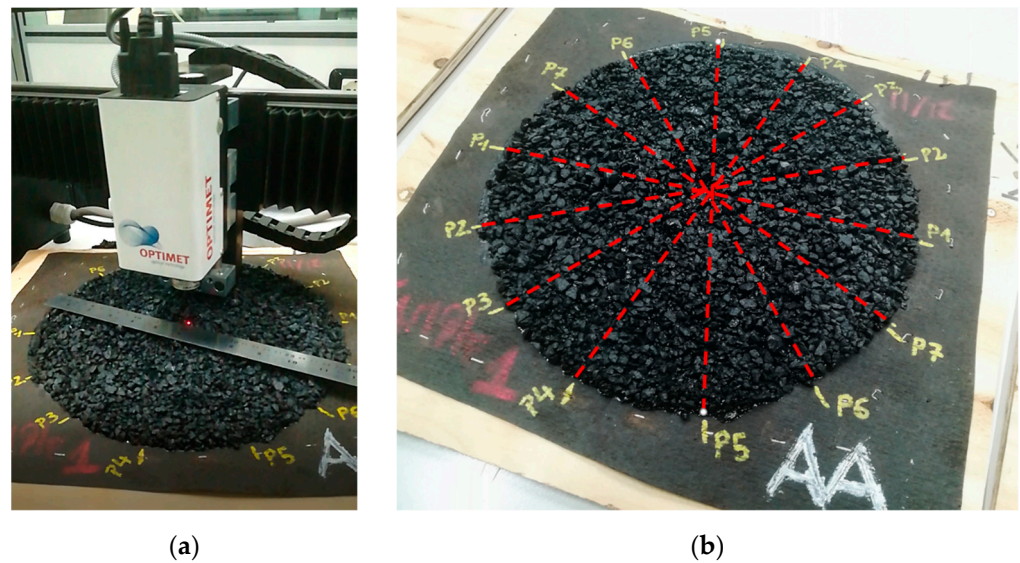


Figure 3. Texture analysis by means of a conoscopic holography laser profilometer: (a) capturing of a profile; (b) alignments identification on a sample.

2.3. Surface Texture Indicators

In general, the surface texture analysis is carried out through different survey methods, with different types of instruments and data post-processing. However, the measurements never exactly match the actual surface, as they represent geometrical entities (succession of heights) that are a discrete approximation of the real surface, and that correspond to a random process. In order for the texture analysis to be significant, the sample must be representative and of an adequate size; in addition, the survey's techniques and data processing must be of an adequate quality. Thus, the characterization of the surface texture must be assumed as a random, stationary, and ergodic signal [47].

Surface texture can be assessed by means of intrinsic criteria, in which the surface geometry is described through the use of discrete functions such as $Z(x,y)$, for a surface, or $Z(x)$, for a profile, or by means of extrinsic criteria, which analyze the surface texture through the use of correlated parameters, such as friction or outflow [48].

For the present study, only intrinsic indicators were used. The latter could be divided into aggregate indicators, which look at the profile as a set of several wavelengths, or spectral indicators, for which the profile is disaggregated by Fourier analysis as the sum of several elementary components, each of which is referred to as a single wavelength λ [49–51].

The methodology used in the surface texture analysis follows the scheme reported in Figure 4; moreover, Section 3, Results and Discussion, is structured following the same scheme.

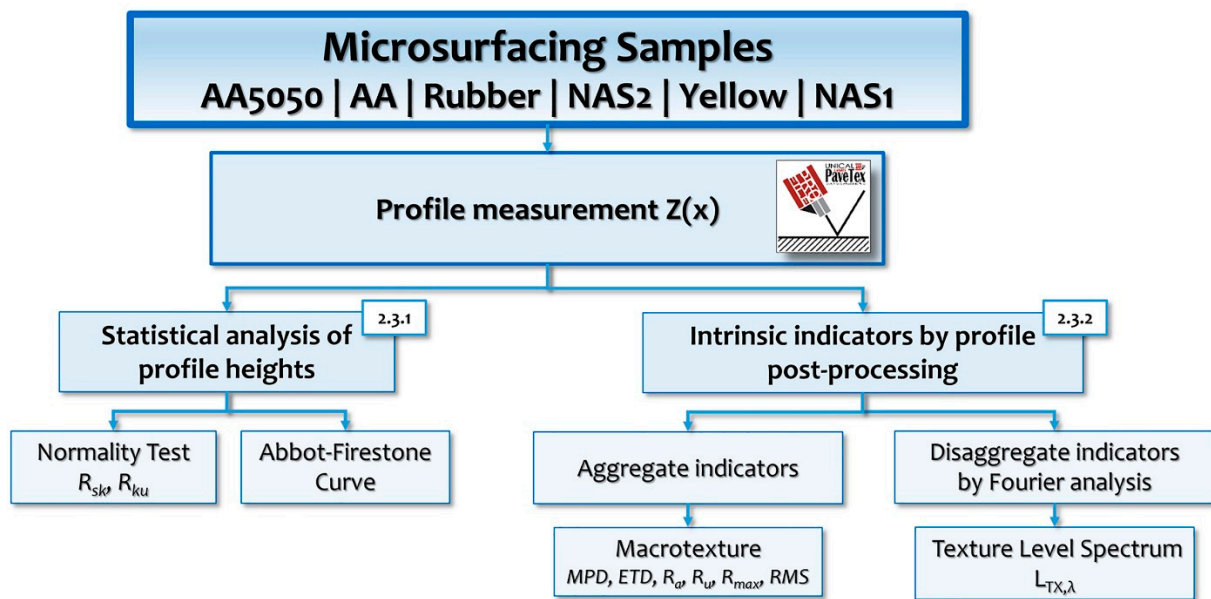


Figure 4. Methodology used for the surface texture analysis.

2.3.1. Statistical Analysis of Profile Heights

The analysis of a surface using aggregated descriptors takes place through simple statistical operations carried out starting from the representative data of the profile, i.e., the discrete function $Z(x)$. Therefore, it is useful to establish the distribution of the heights of the profile by performing a normality test. In addition, it is possible using the provided profiles to calculate some statistical parameters such as the skewness, R_{sk} , the kurtosis, R_{ku} , and the bearing Abbot curve, as presented in Table 3.

Table 3. Statistical parameters of the height distribution.

Indicator	Name	Formula	Graphical Explanation
R_{sk}	Skewness	$R_{sk} = \frac{\sum_{i=1}^n \frac{1}{n} (z_i - \bar{z})^3}{(\sum_{i=1}^n \frac{1}{n} (z_i - \bar{z})^2)^{3/2}}$	
R_{ku}	Kurtosis	$R_{ku} = \frac{\sum_{i=1}^n \frac{1}{n} (z_i - \bar{z})^4}{(\sum_{i=1}^n \frac{1}{n} (z_i - \bar{z})^2)^2}$	
Probability Density Curve	Bearing curve (Abbot curve)	-	

Skewness represents the degree of symmetry of the profile and gives information on how the peaks and valleys are distributed in reference to the profile’s average value.

Skewness assumes a value equal to 0 if the probability density curve of the profile’s height is symmetrical. If the number of peaks above the average value is relatively higher than that of the valley, the skewness assumes a negative value, whereas it will be positive if not.

Kurtosis is a good indicator of the probability density function’s sharpness [52,53]. When Kurtosis is equal to 3 (or 0 if normalized by subtracting 3), the distribution will have the same shape of a Gaussian distribution: where a value of Kurtosis much greater than 3 shows a curve that is really sharp, whereas if it is much lower the curve becomes rounder and flatter.

Thus, when applied to the study of the surface texture, skewness and kurtosis are both parameters that characterize the shape of the probability density distributions of the profile’s height [47].

Finally, the probability density curve, also called the bearing Abbott curve or Abbot-Firestone (or just Abbot) curve, is a parameter usually used in tribology in order to evaluate the functional behavior of surfaces in relative motion subjected to mechanical loading, and it is good indicator of the real area that will support the contact between the surfaces [54–56]. Although the Abbott curve is generally used in mechanical applications, it is possible to extend its use to evaluate the macrotexture of asphalt mix surfaces and so the road–tire contact, with the possibility of inferring drainability and friction properties [2,57]. Mathematically, it represents the cumulative probability density function of the surface’s profile heights, so that, given a certain elevation level, it is possible to have information on the percentage of matter that lies above or below that height. In this context, the following three parameters can be identified on the Abbot curve:

- R_{pk} , which corresponds to the fraction of the profile with the highest peaks and might be linked to the part of the profile that will stick to the tire and be polished under traffic during the pavement service life;
- R_k , which is the contact area region and represents the core of the surface effectively subjected to the traffic loads;
- R_{vk} , which represents the percentage of the profile with the deepest valleys and is perhaps related to the ability of the surface to evacuate or store water [58].

2.3.2. Intrinsic Indicators by Profile Post-Processing

Regarding the aggregate indicators, the ones most used to describe the pavement texture are the mean profile depth (MPD) and the estimated texture depth (ETD) [9,22,59,60]. MPD and ETD are parameters created to assess the pavement macrotexture, and they are based on the traditional sand patch method.

As reported in Table 4, MPD has two different representation. MPDiso is the average value of the profile depth over a certain distance, the baseline, which assumes a value of 100 ± 0.5 mm. Instead, MPDaipcr is the average difference between the regression line of the profile and a line through the top of the highest peak within the sampling length, with a baseline of 138 ± 0.5 mm [9].

Table 4. Intrinsic indicators of the surface texture by profile post-processing.

Aggregate Intrinsic Indicators	Name	Formula	Graphical Explanation
MPDiso	Mean Profile Depth [52]	$MPD = \frac{1^{st} Peak + 2^{nd} Peak}{2} + Average level$	

Table 4. Cont.

Aggregate Intrinsic Indicators	Name	Formula	Graphical Explanation
MPDaipcr	Mean Profile Depth [8]	-	
ETD1995iso	Estimated Texture Depth [18]	$ETD = 0.2 + 0.8 \cdot MPD$	-
ETD2019iso	Estimated Texture Depth [52]	$ETD = 1.1 \cdot MPD$	-
R_a	Average Roughness	$R_a = \frac{1}{L} \cdot \sum_{i=1}^n Z_i - Z_{mean} $	
R_u	Height to the mean	$R_{max} = Z_{max} - Z_{mean}$	
R_z	Average peak-to-valley height	$R_z = \frac{1}{5} \cdot \sum_{i=1}^n Z_{pi} - Z_{vi}$	
$R_{max} (R_t)$	Maximum total height	$R_{max} = \max(Z_{pi}) + \max(Z_{vi})$	
RMS (R_q)	Root Mean Square	$RMS = \sqrt{\frac{1}{n} \cdot \sum_{i=1}^n y_i^2}$	
Spectral Intrinsic Indicators	Name	Formula	Graphical Explanation
$L_{TX,\lambda}$	Texture Level (for given wavelength λ)	$L_{TX,\lambda} = 10lg \left(2 \left \frac{Z_k}{a_{ref}} \right ^2 \right)$ For $k = 0, \dots, \left(\frac{1}{2}N - 1 \right)$	-

ETD is a parameter used to estimate the mean texture depth (MTD) starting from the MPD by using a transformation equation. Thanks to this equation, with ETD it is possible

to approximate the MTD measured with the sand patch method well. The only difference between the indicators ETD1995iso and ETD2019iso is in their formulas [20].

The average roughness, R_a , is one of the most widely used parameters, which gives a gross measure of the surface roughness, and it represents the average height difference (taken in absolute value) in reference to the profile's average line. The parameter R_u , height to the mean or levelling depth, is the height difference between the profile's average line and the maximum profile peak. It gives information on how the peaks diverge from the mean: a low value indicates that the profile is flatter in the part above the average value.

The average peak to valley height, R_z , represents the sum of the mean value of the five tallest peaks and the mean of the five deepest valleys of a profile within the sampling length. R_{max} or R_t is called the maximum total height, and it represents the sum of the maximum peak height Z_p and the maximum valley depth Z_v of a profile within the evaluation length, not sampling length.

The last parameter is the root mean square (RMS or R_q), which is the deviation in height. Some studies have shown that there is an inverse proportion between RMS and friction, either dry or wet [35,61,62].

Finally, regarding the disaggregate descriptors, the texture level, indicated as $L_{TX,\lambda}$ and expressed in decibels (dB), is a logarithmic transformation of an amplitude representation of a surface profile curve $Z(x)$, given a single texture wavelength λ . The texture wavelength describes the horizontal dimension of the amplitude variations of a surface profile and it is expressed in millimeters (mm) [63].

3. Results and Discussions

3.1. Texture Profiles

The contactless laser collected seven profiles for each sample. One profile for each sample was selected as a representative profile, as presented in Figure 5.

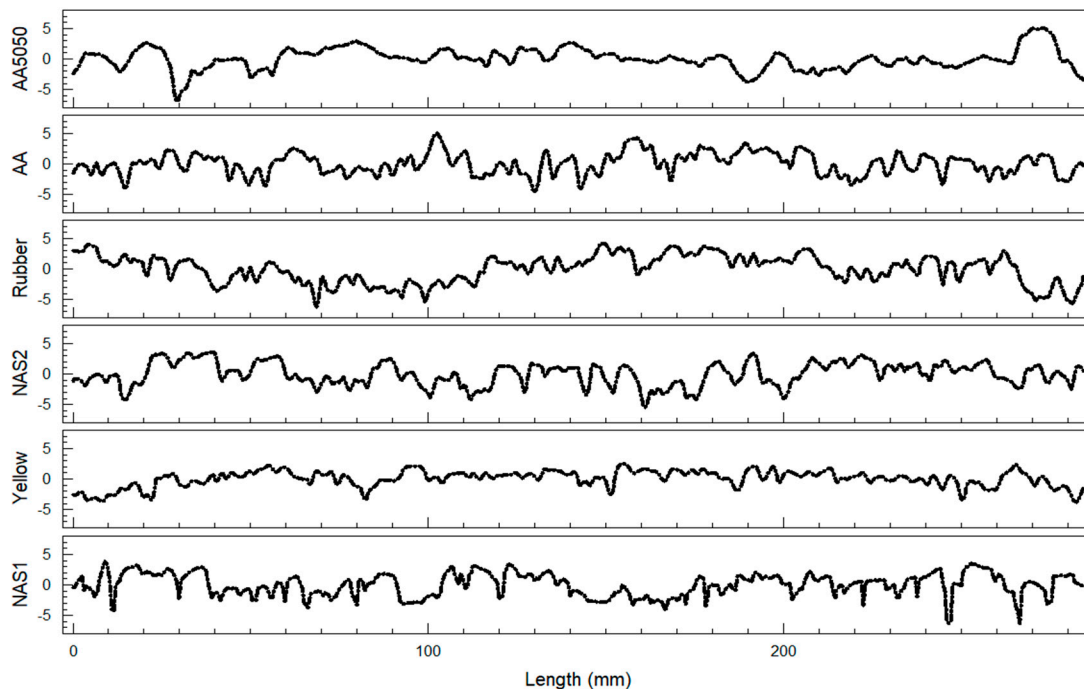


Figure 5. Representative profiles of each sample.

All of the profiles had the same length and were scaled at the same height. It is possible to note that some profiles appeared to be smoother than others, mainly “AA5050” and “Yellow”. However, the profile by itself is not enough to assess and understand the

pavement texture. It is necessary to assess the roughness through the parameters presented in Tables 3 and 4.

3.2. Normality Test

In order to understand how the peaks and valleys are distributed in sample profiles, it is possible to make a comparison using a Gaussian (normal) distribution. The Gaussian distribution has a very interesting property, in that the mean, mode, and median are equal. In Figure 6a, it is possible to see all samples' probability distributions plotted together and Figure 6b the box plot of the same data.

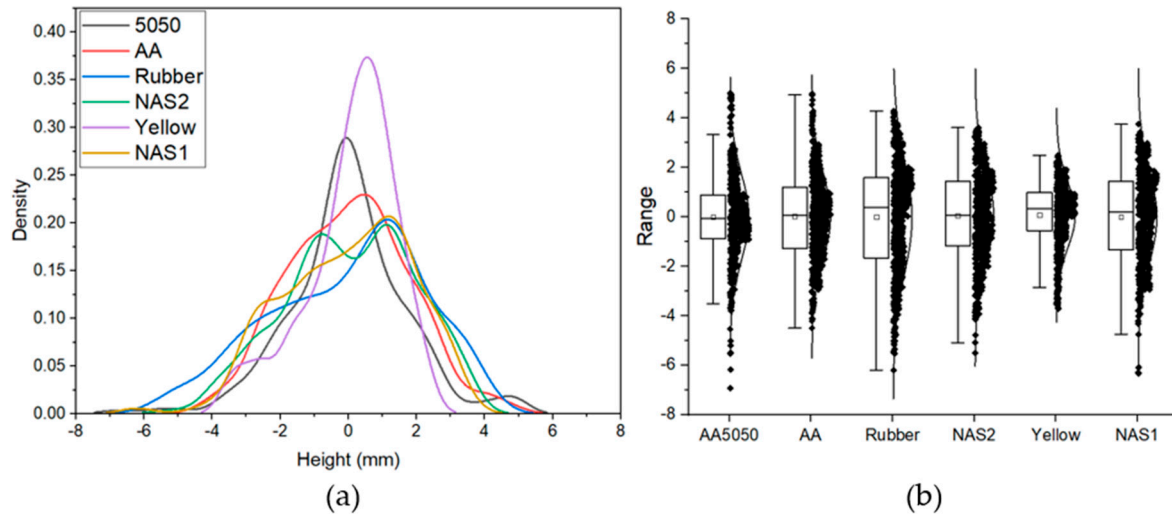


Figure 6. Plot probability density curves (a) and box plot of the samples (b).

For the purpose of assessing if the samples have a Gaussian distribution, we needed to proceed with a specific test. To conduct this evaluation, the Kolmogorov–Smirnov goodness of fit test (KS test) was applied using the Software Origin2021. The KS test compares the data to a normal distribution and calculates the *p*-value and the data statistics. If the former is higher than the *p*-value, the data are not Gaussian (reject normal), this is converse if it is lower. The plotted distribution is in Figure 7, and the data are in Table 5.

Table 5. KS test results and descriptive statistics.

Sample	K-S Test			Descriptive Statistics			
	Statistics	<i>p</i> -Value	Decision at Level (5%)	Mean	Median	Rku	Rsk
AA5050	0.05271	0.02158	Reject normal	0.00258	0.08075	2.03850	−0.38128
AA	0.01551	0.02130	Can't reject normal	−0.00570	0.01032	−0.22259	−0.01487
Rubber	0.04728	0.02140	Reject normal	0.00242	0.17487	0.17292	−0.50802
NAS2	0.03569	0.02099	Reject normal	0.00207	0.04557	−0.37487	−0.18550
Yellow	0.04056	0.02143	Reject normal	−0.00081	0.06757	0.92019	−0.58339
NAS1	0.02559	0.02130	Reject normal	−0.00514	0.01069	−0.33301	−0.15955

The only sample that had a Gaussian-like distribution was sample “AA”. It is possible, through the descriptive statistics in Table 5, to note some relevant differences, the first being that “AA5050” had the highest kurtosis, meaning that the heights were more distributed around the mean value and so it was close to a “flat” surface. This was true also for sample “Yellow”, which shows a quite high kurtosis. In addition, all of the samples had a negative skewness, which means that in all the profiles there is a greater concentration of peaks in respect to the average values.

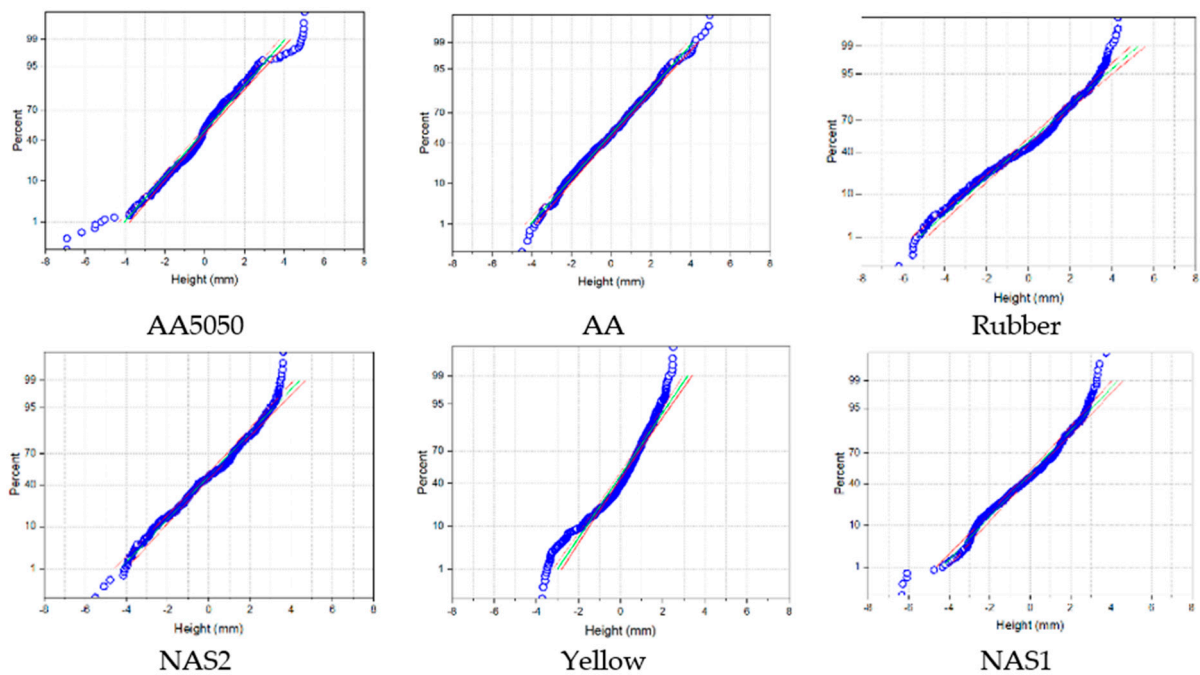


Figure 7. Probability distribution curves.

The reason the samples do not have a Gaussian-like distribution probably depends on the sample’s manufacturing, and on the other hand, as the profiles are conducted radially by the laser profilometer, the sample’s surface finishing is probably affected by the scraping directionality. It could be said that this approach guarantees the stationarity of the signal profile, but not its ergodicity [48].

With the help of a topological graphical representation, a well-established tool in tribology [64,65], it is possible to group and even rank the samples according to their texture properties, as represented in Figure 8.

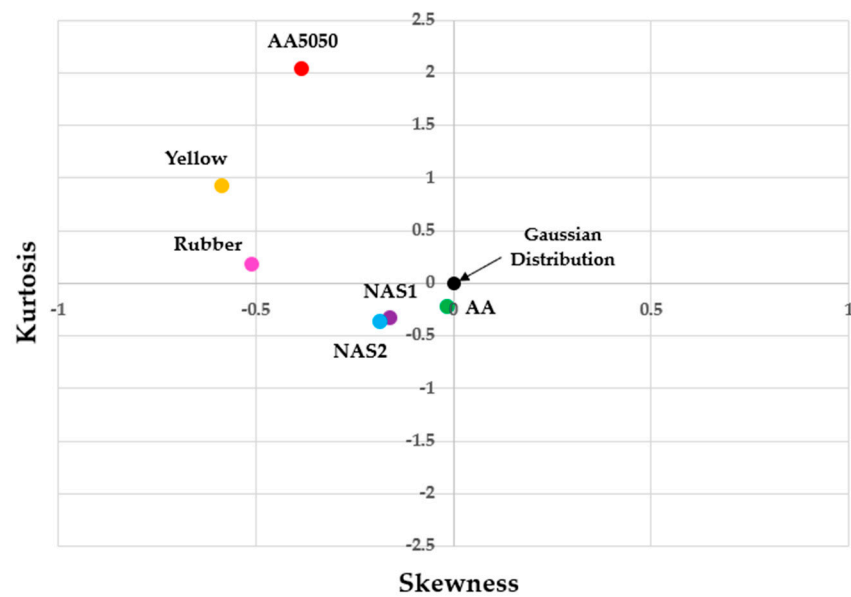


Figure 8. Topological graphical representation.

Sample “AA”, which is close to a normal distribution, has a kurtosis close to zero, although still negative, and a negative skewness. Samples “NAS1” and “NAS2” show quite similar results, with a low negative kurtosis and skewness. However, their profiles cannot

be approximated to a normal distribution. Samples “AA5050” and “Yellow” are similar as they are both positive in the kurtosis. However, they have quite different skewness. The sample “Yellow” has a quite negative skewness but a lower kurtosis, denoting that the data are highly concentrated in the center, with low tails in the distribution. Finally, “AA5050” has a negative skewness and the highest kurtosis.

3.3. Abbott-Firestone Curve

Finally, the last parameters are presented in Figure 9, where the profile is plotted for each sample, followed by the histogram and the distribution, and finally by the bearing Abbot curve.

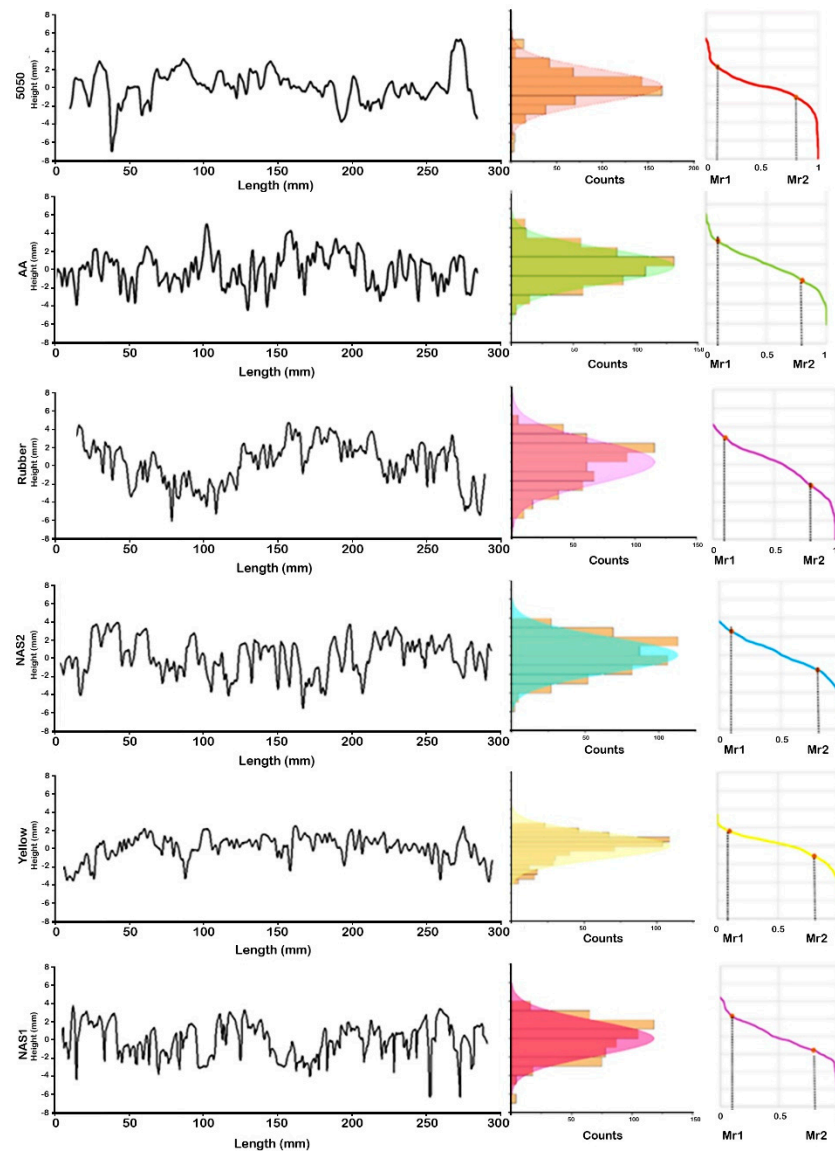


Figure 9. Profiles, histograms, and probability density curve and Abbot-Firestone curves.

The Mr1 and Mr2 points in the Abbott curve represent the height of the profile above or below which a certain portion of peaks or valleys is located, respectively. For this purpose, we chose the peak above which 10% of the profile peaks lay and the valley beneath which 20% of the valleys were included (80% of the whole profile).

The inclination between the Mr1 and Mr2 in the Abbot-Firestone curve (Figure 8) is a good indicator of how the potential contact area of the sample is structured. This contact area can be related to friction, skid, and even noise. An Abbot curve with a steeper

inclination has a less “flat” area, which can indicate that it has a high volume of peaks or valleys. These proportions, peaks, and valleys are given by R_{pk} (peaks) and R_{vk} (valleys), and R_k is the contact area region. This data are presented in Table 6.

Table 6. Abbot curve data and curve inclination.

Sample	Mr1 (mm)	Mr2 (mm)	Inclination (%)	R_{pk} (mm)	R_{vk} (mm)	R_k (mm)
AA5050	2.072	−1.356	4.897	2.996	5.580	3.428
AA	2.381	−1.574	5.649	3.226	4.908	3.954
Rubber	2.500	−1.639	5.913	1.892	3.268	7.098
NAS2	2.391	−1.568	5.655	2.834	5.489	3.959
Yellow	1.511	−0.996	3.581	1.289	4.572	2.507
NAS1	2.392	−1.582	5.678	2.233	6.053	3.974

It is possible to note that the samples “AA5050” and “Yellow”, which were previously cited as being smother due to the other indicators, indeed had a smother inclination, 4.897% and 3.581%, respectively, than the rest of the samples. Samples “AA”, “NAS1”, and “NAS2” showed almost the same inclination and these results were consistent with those found in the topological representation.

3.4. Intrinsic Aggregate Indicators

The texture of the pavement was collected and is presented in Table 7, summarizing the average of seven profiles for each indicator.

Table 7. Average roughness indicators for the whole captured profile.

Indicator	AA5050 (Avg mm)	AA (Avg mm)	Rubber (Avg mm)	NAS2 (Avg mm)	Yellow (Avg mm)	NAS1 (Avg mm)
MPDiso	2.255	3.587	2.631	3.054	1.971	2.930
MPDaipcr	2.776	3.907	2.637	3.568	2.209	3.396
ETD1995iso	2.004	3.070	2.305	2.643	1.776	2.544
ETD2019iso	2.481	3.946	2.894	3.360	2.168	3.223
R_a	1.011	1.499	1.393	1.440	0.835	1.411
R_u	2.564	4.035	2.994	3.584	2.318	3.505
R_z	3.427	5.696	4.463	5.414	3.082	4.617
R_{max}	4.893	6.994	6.318	7.143	4.392	5.935
RMS	1.383	1.850	1.896	1.797	1.108	1.776

It is noted that the lowest MPD was from the “Yellow” mix, followed by “AA5050”, with the highest values found for “AA” and “NAS2”. It is clear that the smother surfaces, indicated in Figure 5, were indeed “Yellow” and “AA5050”, as indicated by R_a close to 1 and the lowest RMS values. “NAS1” and “NAS2” had quite similar values in all indicators, with the exception of R_{max} . This could be due to a lack of homogeneity in the surface, as they were the samples manufactured with the same mixture.

Finally, by plotting the relationships between the MPD values and roughness indicators, as shown in Figure 10, it was seen that they were well-correlated, and thus it was possible to use any of them as a general indicator. Either way, they had different meanings and conveyed different information.

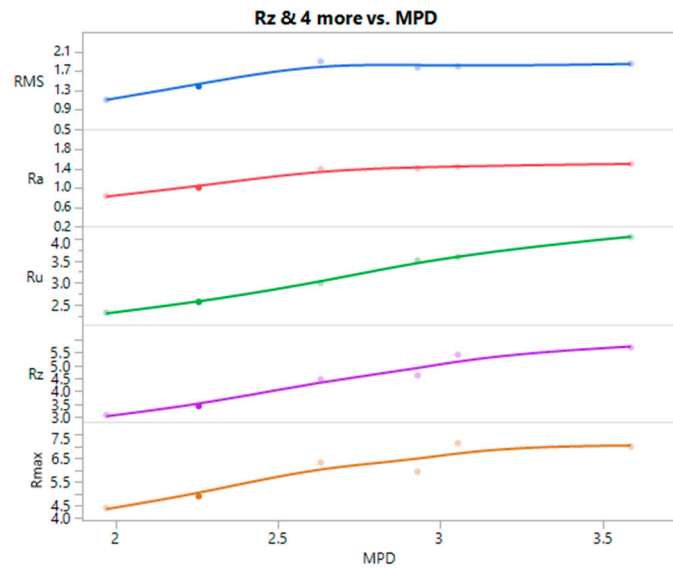


Figure 10. MPD by other height indicators.

3.5. Texture Level Spectrum

In Figure 11, it is possible to observe the texture level spectrum of the different microsurfacing samples, which shows the texture level (L_{TX}) for a certain class of wavelengths λ , from 0.063 mm to 20 mm. In particular, it is possible to observe the microtexture domain for wavelengths below 0.5 mm, whereas the macrotexture domain ranges from 0.5 mm to 50 mm [47,49].

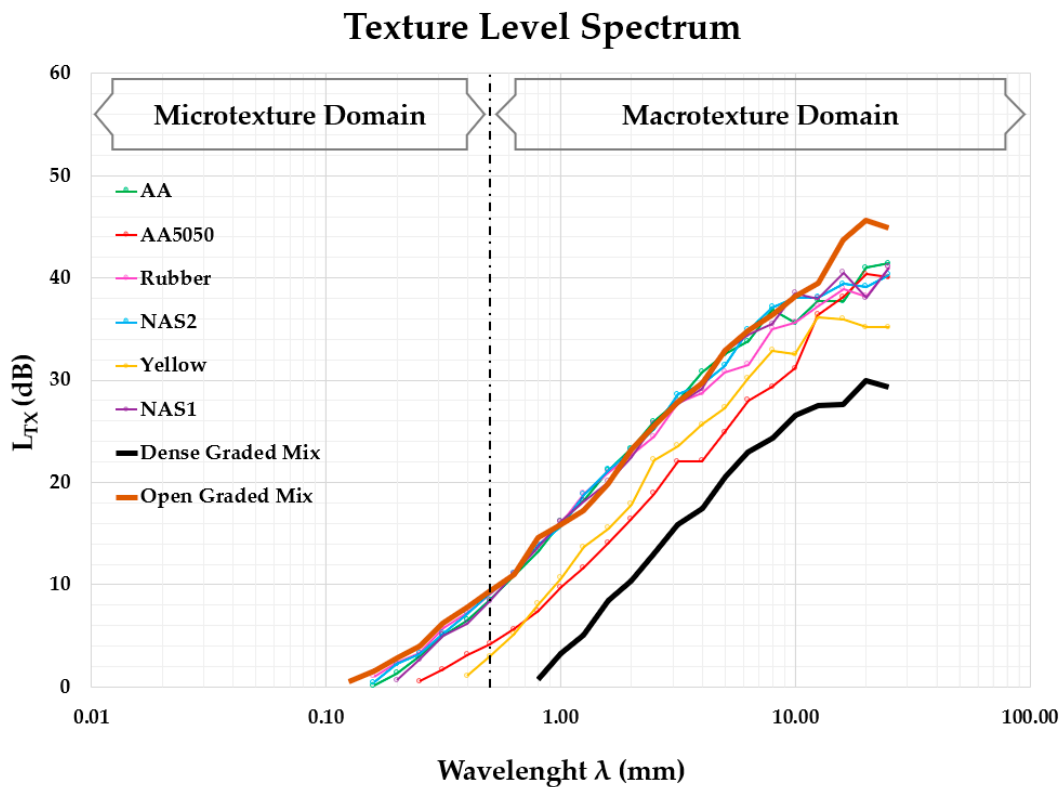


Figure 11. Texture Level Spectrum of the samples.

For comparative purposes, the microsurfacing samples were compared to an on-site application of an Open Graded (OG) mix and a Dense Graded (DG) mix. The OG texture

data were acquired at the time of traffic opening, whereas the DG texture data were deliberately taken after conditioning by traffic. The OG mix used basalt with a maximum aggregate size between 15 to 20 mm, whereas DG was a mixture made up of basalt and limestone with a 0/8 granulometric curve.

As for the OG spectra, it was noted that at a larger wavelength, the texture level was higher compared with the other mixtures, and this trend was also true in the microtexture domain. The DG spectra lay below all of the mixtures: it was clear that the traffic action exposed the surface to raveling, which led to a loss of aggregates and thus to a reduction in the texture level in the macrotexture domain, and polishing, which explains the complete lack of microtexture. In conclusion, the OG and DG spectra appeared to include the microsurfacing, indicating that microsurfacing is a maintenance solution that could offer a texture level between them.

As for the microsurfacing samples, it was clearly noticeable that all of the mixtures had similar results, except for the "AA5050" and "Yellow" samples; these last ones were placed lower down in the spectra. This can be explained by comparing these results with the granulometric distribution of the samples. In fact, "AA5050" had a particle size obtained by combining 50% of a 0/6 granulometric curve passing sieve 2# and 50% AEA of 10 mm in size, whereas the "Yellow" sample was made up with 0/6 mm. Therefore, this explains the similar trend until the wavelength of 2 mm was reached, then there was an upturn of the "Yellow" curve due to the lack of particle between 2 and 10 mm in the "AA5050" mixture. Lastly, the "AA5050" curve rose due to the presence of AEA, whereas the "Yellow" curve reached a plateau.

The other mixtures' spectra followed the same trend, especially at shorter wavelengths. In fact, there were no major differences between the 0/8 mm granulometric distribution ("NAS1" and "NAS2") and the Australian curve ("AA"). In addition, the "AA" sample did not show particular differences, despite the presence of AEA, because of the small percentage used in the mixture. The spectra of "Rubber" exhibited a lower texture level between wavelengths of 4 and 10 mm. Finally, it could be said that the small difference between the last discussed mixtures could be attributable to the way the mixtures were laid, and were not statistically relevant.

By looking at the microtexture domain, it seems that the texture level of each sample was outlined. The microtexture in the "Yellow" sample did not emerge, probably due to the use of resin, which covered the roughness of the single aggregate itself. The "AA5050" sample showed a positive level of texture until reaching a 0.25 mm wavelength, but the microtexture was still more hidden than that of the rest of the mixtures. This could be attributable to the shape and the surface finish of AEA, considering that they were casted in molds.

These results might be related to the inclinations found in the Abbott curves; actually, the microtexture was obtained by the surface properties of the individual chippings or other small particles of the surface that come into direct contact with the tire [21], and thus the core of the surface. In fact, as previously noted, the "Yellow" and "AA5050" samples had a smoother surface than the other samples.

4. Conclusions

The present paper aimed to assess and evaluate the texture of different microsurfacing samples. The proposed work consisted of testing two benchmark mixtures that used a traditional mix design made with natural aggregates, one with asphalt emulsion and the other with a resin binder. Different mixtures were tested in comparison: a mixture with asphalt emulsion, 60% crumb rubber and 40% natural aggregate, and two mixtures with artificial geopolymer aggregates, with different granulometric curves.

The analysis carried out in the present work showed microsurfacing to be a maintenance solution that, depending on the mix design composition (i.e., particle size and amount of bitumen emulsion), give a texture spectrum level that lies in a range between a

dense graded and an open graded asphalt mix. It will be the designer's responsibility to decide the mix design in order to reach the desired surface texture performances.

Moreover, it is possible to conclude the following:

- Microsurfacing is confirmed to be a preventative maintenance solution that can restore the pavement surface, providing good texture properties;
- The texture parameters, traditional or statistical, are dependent on the type/size of aggregates and type/proportion of the binder used; similarly, it is noted that the samples' manufacturing and profile's survey directionality could also impact them;
- Based on the adopted mix design, the use of crumb rubber as an aggregate, even at a smaller nominal size, did not present relevant differences in terms of the texture indicator when compared to the other mixtures with the same granulometric distribution;
- The use of AEA has been shown to have an influence on the texture indicators, probably due to its size, 10 mm, but also due to its shape;
- The authors believe that it is necessary to reproduce these tests with a more diverse sample scenario, using different binders and aggregates, while also including information on skid resistance.

In conclusion, the use of topological maps to describe and classify the surfaces according to skewness, kurtosis, and the Abbot curve could provide interesting information as it is widely used in tribology. Therefore, the use of the well-known and established texture parameters, supported by such a statistical analysis, could be useful in the laboratory phase to manage the resulting surface texture of microsurfacing and, to some extent, to explain the tire/pavement interaction, the rolling resistance offered by the surface, and the polishing action due to vehicles.

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