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Determination of the dispersion relation in cross-laminated timber plates: benchmarking of time- and frequency-domain methods

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Abstract

The availability of a reliable estimate of the dispersion relation of plates is of great importance for acoustic modelling. Several experimental methods for the extraction of wavenumber and wave velocity information are discussed in the literature. They differ, among other factors, by the sensitivity to noise and multiple reflections, frequency range of application and implementation of the processing algorithm. While for homogeneous thick or thin plates well-established analytical solutions are available, non homogeneous materials would benefit from experimental characterisation. In this framework, our attention is focused on cross-laminated timber (CLT) elements, load-bearing orthotropic layered wood plates that revolutionised the sector of timber construction. The anisotropic and non homogeneous nature of wood suggested testing methods that, though converging for homogeneous materials, could bring different results in their application to CLT elements. The aim of this paper is therefore to benchmark methods for the extraction of wavenumber information on CLT plates, **from which the effective elastic properties to model CLT structures as an equivalent homogeneous plate can be derived**. This is achieved through a numerical and experimental benchmark of well-established methods for the evaluation of the real component of the propagating wavenumber in CLT elements. Five relevant methods were selected through a literature review, implemented and analyzed: three in the time domain, namely maximum peak, cross-correlation and kurtosis, and two in the frequency domain: phase difference and wave correlation. First, the five methods were benchmarked by numerical simulation on a homogeneous material. In this ideal case, results showed to be consistent for all methods. Then, wavenumber measurements were performed on a cross-laminated timber plate. The results show that the maximum peak, cross-correlation and the wave correlation method provide the lowest dispersion of the data. Considering the time required by the installation of the setup,

the wave correlation method seems to be the best alternative among the proposed ones.

Keywords: Structural acoustics, wave propagation, dispersion relation, cross-laminated timber

1. Introduction

Cross-laminated timber (CLT) is an engineered-wood product made of layers of timber planks glued crosswise. The product was originally developed in the late 1990s with the aim of using cuts that could not be used otherwise for structural products and it integrates the set of available wood engineered products [1]. CLT elements are bi-dimensional products characterised by a high out-of-plane stiffness; the alternation of crossed layers also allows to compensate for shrinkage effects, which are much more evident in products made of layers with equally oriented strands. The diffusion of CLT has allowed the wood construction market to develop integral timber construction systems also for large volumes/high rise constructions. In hybrid high-rise timber constructions, CLT is used for floors, shear walls and vertical-loads carrying elements. In-depth research has been conducted to investigate seismic behaviour, thermal insulation and fire resistance of CLT elements [2, 3, 4, 5]. The evolution of the construction sector, moving towards prefabrication, is seeking solutions for shortening construction time and higher quality standards. Thanks to the reduced weight, wood elements are a natural choice to produce highly prefabricated components and the whole wood construction market is expected to see a relevant increase in volume in the forthcoming years [6]. Timber construction plays a relevant role in meeting the goal of decarbonisation of the construction sector. Its eco-friendly environmental footprint can be traced back to several factors: first, the production of the raw material has a low environmental impact compared to traditional construction materials - also without considering the CO₂ stored by the trees during growth [7]. Then, a turning point is represented by the transition from the concept of “extraction” to the concept of “growth” of the natural resource [8]. Finally, another environmental benefit associated with the construction process of timber buildings is represented by the reduced emission of particulate matter during construction [9].

Acoustics, ultrasonic, and vibro-acoustics measurement techniques are widely used throughout the whole wood value chain for the estimate of the elastic properties of wood and for the detection of damage. Sonic and ultrasonic testing, together with vibration-based testing, are techniques commonly used to assess properties of timber elements (such as MOE and moisture content), or the presence of damage such as delamination or defects [10, 11, 12, 13]. In particular,

29 ultrasonic testing is one of the most common methods used for timber grading (see, for instance,
30 [14]), together with modal testing (see, for instance, [15]). As previously mentioned, CLT el-
31 ements in high-rise multi-unit residential buildings built using a hybrid building technology are
32 mainly used to load bearing walls and floors. It is then straightforward that an in-depth knowl-
33 edge of acoustic modelling of sound radiation of such elements is important to guarantee the
34 health and comfort requirements in terms of sound insulation. Sound radiation is a phenomenon
35 associated to the vibration of a plate coupled with wave propagation in the adjacent fluid, air in
36 our case. Radiation is governed by flexural waves, which are dispersive: the evaluation of the
37 structural wavenumbers freely propagating in a given structure is a key information regarding its
38 dynamic behaviour. ~~The accurate estimate of the structural wavenumber also allows for the iden-~~
39 ~~tification of the coincidence frequencies, and therefore to distinguish between the three regions~~
40 ~~marked by the critical frequency, characterised by different radiation efficiencies.~~ Assuming the
41 equation of motion of the structure to be known, the experimental wavenumber, or wave velocity
42 dispersion relation, allows for a complete characterisation of the elastic and damping properties
43 of the material [16]. **The wavenumber dispersion relation can be used to identify the elastic con-**
44 **stants of a homogeneous element, or to determine a set of apparent effective elastic properties,**
45 **which can also be frequency-dependent, of a structure with a rather complex dynamic behaviour**
46 **(eg. such as laminated or sandwich elements) [17, 18], in order to approximate its vibro-acoustic**
47 **response by using the simple theories for thin or thick plates.**

48 ~~The flexural wavenumber can be modelled or measured.~~ Several experimental methods ex-
49 ist to extract wavenumber information, which, according to Margerit et al. [19], can be clas-
50 sified in relation to four features: robustness, frequency range of application, ability to detect
51 angle-dependent features and ability to return real or complex wavenumbers. Another factor
52 of discrimination is the input data required for the implementation of each method. The ease
53 of the measurement procedure is also a factor to consider [20]. While dealing with ideal, ho-
54 mogeneous and isotropic plates, modelling and measurements converge. Also the differences
55 between different measurement and signal processing methods agree. Conversely, dealing with
56 non homogeneous and non-isotropic materials, discrepancies arise between modelling and mea-
57 surements, and also between measurement techniques. For what concerns CLT structures, both
58 resonant and broad frequency-based, as well as time-based methods to investigate their elastic
59 properties, can be found in the literature. For example Steiger et al. [21] and Gsell [22] et al. pre-

60 sented experimental methods to determine the elastic properties of CLT structures considered as
61 an equivalent homogeneous orthotropic panel. Van Damme et al. [23, 24] performed experimen-
62 tal testing on CLT beams in order to determine structural elastic properties through a comparison
63 of the experimental bending resonance and value resulting from either the classic theory for thin
64 beams or the Timoshenko theory for thick beams. Santoni et al. [16] proposed an experimen-
65 tal method based on wave velocity identification to determine the frequency-dependent elastic
66 properties of a CLT plate, by evaluating either the time delay or the phase difference between a
67 pair of signals. A quick procedure to extract wavenumber information and thus to retrieve the
68 elastic constants of CLT panels was presented by Thies et al. [25], showing preliminary results
69 of an experimental analysis conducted on several CLT panels, characterised by different thick-
70 ness and orthotropic ratios. These results were compared with the elastic parameters provided
71 for structural calculations.

72 This paper does not want to provide an extensive literature review of the different non-
73 destructive methods available to investigate the elastic properties of CLT elements. It rather
74 aims at identifying which methods are more suitable to be used for the assessment of flexural
75 wavenumber propagation, **within a frequency range adequately extended in order to evaluate**
76 **effective elastic properties of CLT elements**, which are inhomogeneous, non-isotropic layered
77 structures. Five representative methods which can be applied within the acoustic frequency, were
78 firstly benchmarked using a numerical dataset obtained from Finite Element (FE) simulations on
79 an isotropic homogeneous structure. Then they were applied for the experimental evaluation of
80 the dispersion curves of a CLT panel.

81 The paper is organised as follows. The next section expresses the motivation for this work,
82 and why it is often necessary to adopt homogenised models for a vibro-acoustic analysis of CLT
83 systems. The theoretical background on bending wave propagation for thin and thick plates is
84 provided in Section 3, together with the description of the methods considered in the present
85 study. Section 4 benchmarks the different methods using numerical input data, to assess how
86 each method performs under optimal conditions. Section 5 presents the test facilities and the
87 techniques used to acquire the experimental input data. The data processing procedure and the
88 experimental results are presented in Section 6. For each method, the outcomes of the exper-
89 imental application are analyzed in relation to the numerical one. Furthermore, strengths and
90 weaknesses of the methods are discussed. The main outcomes of this work are finally sum-

91 marised in the conclusions.

92 2. Motivation and method

93 Knowledge of dispersion relations of CLT building elements is relevant for the modeling of
94 their vibro-acoustic behavior as it allows to estimate the plate's elastic properties, damping, and
95 coincidence frequencies. CLT proves to be a challenging benchmark for comparative testing be-
96 cause of the peculiar mechanical properties of wood. Wood is an anisotropic and inhomogeneous
97 material; CLT panels, due to their production process, very often exhibit a highly orthotropic
98 behaviour. Several attempts to model the structural and the vibro-acoustic behaviour of CLT
99 elements can be found in the literature. Qian et al. [26] recently presented an FE-based predic-
100 tion tool to quantify the uncertainty induced in vibro-acoustic modelling of CLT structures by
101 the material properties. Due to the complexity of CLT systems, they were often approximated
102 to an equivalent homogeneous element. For example Fürtmüller et al. [27] proposed an FE ap-
103 proach to derived equivalent mechanical properties to investigate the structural behaviour of CLT
104 components treated as shell elements. Santoni et al. [28, 29] presented prediction methods to
105 evaluate the resonant radiation efficiency and the transmission loss of CLT panels considered as
106 an equivalent thin orthotropic plate and characterised by frequency-dependent elastic properties.
107 Higher order plates theories were considered in order to improve the modelling of shear defor-
108 mation, both from a structural point of view [30], as well as a vibro-acoustic one [31]. Yang et
109 al. [32] recently presented a wave and finite element approach to investigate the vibro-acoustic
110 behaviour of CLT panels either by modelling each layer as a homogeneous orthotropic material,
111 or by considering the panel as a single layer of an equivalent homogeneous orthotropic material.
112 The identification of wavenumber dispersion relation of a CLT panel allows to obtain informa-
113 tion about the dynamic behaviour of the structure, which can be used either to describe it as an
114 equivalent homogeneous element, or to determine its elastic properties through a best fitting of
115 the experimental data with an analytical dispersion relation. The identification of the wavenum-
116 ber dispersions relation of a CLT panel is relevant for its vibro-acoustic modelling. In fact, these
117 can be used to evaluate effective elastic properties to treat the CLT structure as an equivalent
118 homogeneous element, and to approximate its vibro-acoustic behaviour by means of simplified
119 plates theories. The accuracy of vibro-acoustic simulations based on either the classical plate,
120 or the first order shear deformation theories, rather than on the three-dimension elasticity model,

121 were discussed by Arasan et al. [33]. As already mentioned, this contribution aims at providing
 122 a comparative assessment of five measurement methods, which, according to the authors, are
 123 among the best suited for the identification of the dispersion curve of CLT structures, within the
 124 acoustic frequency range. This will be useful for researchers and engineers looking for a suitable
 125 tool to determine the effective elastic properties of CLT structures, to be used as input data in
 126 vibro-acoustic simulations.

127 Methods implemented in the time domain and in the frequency domain were chosen, re-
 128 quiring different measurement setups, showing different computational times and robustness to
 129 non-ideal propagation characteristics of media. The comparison was first conducted through a
 130 benchmark on numerical simulations performed with FE models. Then, the same methods were
 131 tested on experimental measurements performed on a CLT plate. The investigation therefore
 132 seeks to benchmark different methods for the extraction of wavenumber information, whose re-
 133 liability was previously assessed on a numerical data set, through the use of real measured data
 134 with the aim of identifying the algorithm and measurement technique that is better suited for
 135 CLT elements. Through a thorough analysis of the outcomes and drawbacks of each method,
 136 the results mean to provide indications on how to approach these structures and to reduce the
 137 uncertainties that wrong estimates of k have on calculation methods.

138 3. Theoretical background

139 In classical plate theory, there is a distinction between ‘acoustically’ thin and thick plates.
 140 When $\lambda_B > 2\pi h$, being λ_B the flexural wavelength and h the thickness of the plate, shear defor-
 141 mation and rotational inertia can be neglected and the plate can be modeled as thin. Thin plates
 142 are modeled according to the Kirchhoff-Love theory [34] and the dispersion relation follows:

$$143 \quad c_K = \sqrt{\omega}^4 \sqrt{\frac{Eh^2}{12\rho(1-\nu^2)}} \quad (1)$$

144 where c_K is the bending wave velocity (thin plates), ρ is the density of the plate, h its thickness,
 145 ω is the angular frequency, E the elastic modulus and ν Poisson’s ratio.

146 The formulation of the governing equation for thick plates, accounting for shear deforma-
 147 tion and rotatory inertia, was derived in the 1950s by Mindlin. The dispersion relation hence
 148 becomes [35]:

$$149 \quad \left(1 - \frac{c_M^2}{T^2 c_S^2}\right) \left(\frac{c_L^2}{c_M^2} - 1\right) = \frac{12}{h^2 k_K^2} \quad (2)$$

150 where c_M is the bending wave velocity (thick plates), c_S is the shear wave velocity, c_L is the
 151 longitudinal wave velocity, h is the thickness of the plate and k_K is the wavenumber computed
 152 from the bending velocity given in Eq. (1).

153 To consider that the shear stress is not constant over the plate cross-section, the coefficient T
 154 is given as the ratio between the Rayleigh surface c_R wave velocity and the shear wave velocity
 155 c_S , and can be computed as [36]:

$$156 \quad T = \frac{c_R}{c_S} = \frac{0.87 + 1.12\nu}{1 + \nu} \quad (3)$$

157 The longitudinal and the shear wave velocities can be described as:

$$158 \quad \begin{aligned} c_L &= \sqrt{\frac{E}{\rho(1-\nu^2)}} \\ c_S &= \sqrt{\frac{G}{\rho}} \end{aligned} \quad (4)$$

159 where G represents the shear modulus.

160 For thin plates, the velocity of bending waves is proportional to the square root of the fre-
 161 quency. The dispersion curve for thick plates follows an asymptotic behavior; at low frequencies
 162 it behaves according to the thin plate theory, while at higher frequencies it approaches asymptot-
 163 ically the velocity of shear waves. Rindel [37] provided a simplified relation that describes the
 164 propagation of bending waves as a combination of the velocity of bending waves according to
 165 Kirchhoff-Love theory and the velocity of shear waves:

$$166 \quad c_{B,eff} \approx \left[\frac{1}{c_K^3} + \frac{1}{c_S^3} \right]^{-1/3} \quad (5)$$

167 From the wavenumber dispersion curve it is possible to evaluate the elastic properties of the in-
 168 vestigated structure by assuming an analytical dispersion relation. A non-linear fit of the exper-
 169 imental wavenumbers, with the chosen dispersion relation, would provide the elastic properties
 170 of the investigated structure. When rotatory inertia and shear deformation have a negligible in-
 171 fluence on the structural response, the wavenumber dispersion relation based on Kirchhoff's thin
 172 plate theory can be derived from Eq. (1). However, when the structural wavelength is of the
 173 same order of magnitude as that of the thickness of the structure, shear deformation and rotary
 174 inertia have to be considered, otherwise the results will be misleading. In this case, Mindlin's
 175 wavenumber dispersion relation for thick plates should be used. This is analogous to Eq.(2),
 176 which express the dispersion relation in terms of wave velocity. By using Eq.(2), expressed in
 177 the wavenumber domain, it is possible to determine both the modulus of elasticity E and the

178 shear modulus G , when the latter affects the dynamic behaviour of the investigated structure;
179 otherwise, if the influence of shear deformation is negligible, the fitting procedure would still
180 provide an accurate elastic modulus E even though a non reliable shear modulus G would also
181 be found, as discussed in the next section.

182 The methods that can be used for the determination of the wavenumber can be categorised
183 as time-domain methods and frequency-domain methods. In the following, the five methods
184 considered are briefly described.

185 *3.1. Time domain methods*

186 Time domain methods are based upon the determination of the Time-of-Flight (ToF), i.e.
187 the time lag characterizing the acquisition of a signal by two sensors separated by a known
188 distance. Natural applications of these methods are in the mid-high frequency range, because
189 of the limitations arising due to the distance between the sensors in relation to the investigated
190 wavelength. In fact, time domain methods are commonly used in the ultrasonic frequency range,
191 where the accuracy of the onset detection is of paramount importance [38]. The velocity of wave
192 propagation in the time domain is usually measured through the analysis of a wave packet in
193 order to minimise the dispersion effects and to easily window out unwanted reflections.

194 *3.1.1. Maximum peak*

195 The most common method for the determination of the ToF is the identification of the max-
196 imum peak of the signal. Considering a pulse centered on a known frequency and recording
197 the signal at two positions, aligned with the source and having a mutual distance d , the time
198 signal recorded by the second accelerometer will be attenuated and time-delayed with respect
199 to the first one. Identifying the arrival time of the absolute maximum of the pulse at the two
200 positions, the time difference t will provide a direct estimate of the ToF [39]. The velocity of the
201 propagating wave c can be finally evaluated through the relation $c = d/t$. If multiple pulses are
202 generated at different frequencies, the dispersion curve can be built frequency by frequency. The
203 frequency range of application of the ToF algorithm based upon the detection of the maxima is
204 related to the distance between the receivers. At low frequencies, when the wavelength is greater
205 than the distance between the accelerometers, this method cannot be used. For low frequencies,
206 the dispersion curve is often calculated with the phase difference between the two signals. The

207 frequency limit that devises the phase and the ToF approach cannot be calculated *a priori* be-
 208 cause the phase velocity is unknown. Thus, both methods are often implemented on overlapping
 209 frequency ranges around the guessed limit frequency. Most of the limitations related to the use
 210 of this method are the robustness problems arising when the signal-to-noise ratio (SNR) is low
 211 and when multiple reflections generate interference.

212 3.1.2. Cross-correlation

213 Cross-correlation is another well-known technique for the estimation of the ToF. Two signals
 214 s_1 and s_2 are recorded by accelerometers spaced at a distance d . The signals are cross-correlated,
 215 i.e. they are shifted one with respect to the other and their product is summed term by term:

$$216 \quad (s_1 \star s_2)(\tau) = \int_{-\infty}^{\infty} s_1^*(t) s_2(t + \tau) dt \quad (6)$$

217 where $*$ denotes the complex conjugate. When the signals are aligned, the cross-correlation
 218 function assumes a maximum at a lag τ that corresponds to the ToF. Dividing the distance d by
 219 the ToF, a direct estimate of the wave velocity can be obtained [40]. The accuracy of this method
 220 depends upon the width of the correlation peak, providing better estimates when narrow. This
 221 method is quite sensitive to multiple early reflections [41], therefore only the direct field should
 222 be analyzed. The correlation approach is also at the base of the Wigner-Ville analysis, used to
 223 extract wavenumber information [42].

224 3.1.3. Kurtosis

225 The kurtosis is the fourth order cumulant of a distribution; it measures how much the proba-
 226 bility distribution of a variable differs from a normal distribution. Since the kurtosis of a normal
 227 distribution equals 3, it is common to describe the normalised kurtosis as:

$$228 \quad \mathcal{K} = \frac{X(x - \mu)^4}{\sigma^4} - 3 \quad (7)$$

229 where X is the expected value, μ is the mean value of the series and σ is the standard devia-
 230 tion. Assuming that measured noise is Gaussian, the normalised kurtosis calculated on a running
 231 window will assume a value of zero. When the direct sound arrives, the tail of the distribution
 232 changes rapidly and the kurtosis increases rapidly. Thus the arrival of the direct sound is related
 233 to the point in which the kurtosis reaches a maximum. Thus, taking the corresponding time of the
 234 direct path, the phase velocity can be evaluated [43]. Unlike classical ToF methods, a high order

235 cumulant such as the kurtosis contains amplitude and phase information and therefore provides
 236 an interesting advantage in the determination of the onset of a signal [44]. For the application of
 237 the kurtosis method, the most relevant step is the correct choice of a suitable time window to run
 238 over the signal and on which to compute the kurtosis.

239 3.2. Frequency domain methods

240 Several alternative methods can be used in order to characterise elastic or visco-elastic ma-
 241 terials [45], which, unlike the approaches introduced so far, are applied in the frequency domain
 242 rather than in the time domain. While time domain algorithms generally use single frequency
 243 pulses, frequency domain measurements are generally performed using broad band exciting sig-
 244 nals. Some of them, based on resonant techniques, allow to evaluate the elastic properties associ-
 245 ated to the resonant frequency of the measured dynamic response [46, 23], while other methods
 246 allow to directly determine the dispersion curve either in terms of the propagating wave velocity
 247 or in terms of wavenumbers [24, 47].

248 3.2.1. Phase Difference

249 The phase velocity of lateral displacement can be determined by evaluating the phase dif-
 250 ference between a pair of adjoining transducers. Therefore, even though this method deals with
 251 signals in the frequency domain, its application is very similar to the time-domain based methods
 252 previously described. The dynamic response of the investigated structure is in fact evaluated on
 253 pairs of positions. A fast Fourier transform (FFT) is performed on the signals, conveniently win-
 254 dowed to consider only the direct incoming wave [48], in order to obtain magnitude and phase
 255 in the frequency domain. The real part of the flexural wavenumber $\text{Re}(k_B)$ can be evaluated and
 256 averaged over n measurement positions as:

$$257 \quad \text{Re}\{k_B(\omega)\} = \frac{1}{n-1} \sum_{i=1}^{n-1} \left(-\frac{\Delta\phi_{i,i+1}}{\Delta d_{i,i+1}} \right), \quad (8)$$

258 where $\Delta\phi$ represents the phase difference between two measured signals and Δd the mutual
 259 distance between corresponding sensors.

260 3.2.2. Wave Correlation method

261 Wave-correlation approaches require the evaluation of the dynamic response of the investi-
 262 gated structure, usually due to a broadband excitation, along a line, or over a grid of equally

263 distributed points. A spatial Fourier transform (SFT) can be applied to the dynamic response
 264 $w(\omega, x)$ in order to obtain its spectrum in the wavenumber domain $w(\omega, k_x)$.

265 The length L_e over which the structure dynamic response is measured determines the lower
 266 frequency limit for wavenumber extraction, according to the relation: $L_{e,min} \geq \lambda(f_{min})/2$. The
 267 spacing between measurement points determines the maximum wavenumber that can be evalu-
 268 ated according to the Shannon-Nyquist theorem criterion: $k_{max} < \frac{\pi}{2\Delta x}$.

269 When the investigated structure has clamped boundary conditions, a zero-padding is usually
 270 applied to the measured response, artificially increasing the wavenumber resolution [24, 49].
 271 Alternatives to the SFT, to perform transformation of the measured structural response from the
 272 spatial domain to the wavenumber domain, were discussed by Roozen et al. [50].

273 For example, a correlation function can be computed between the measured dynamic re-
 274 sponse $s(\omega, x)$ and a propagating inhomogeneous plane wave $o(\omega, k_x)$, as proposed by Berthaut
 275 et al. [51] and by Ichchou et al. [52, 53].

276 The 1D inhomogeneous plane wave is defined as: $o(\omega, k_{x,r}, k_{x,i}) = \exp(\pm jk_{x,r}x + k_{x,i}x)$,
 277 where $k_{x,r}$ is the real part of the complex wavenumber \tilde{k}_x , while $k_{x,i}$ represents its imaginary
 278 component, associated with the damping of the system. In this study, a mono-dimensional plane
 279 wave was considered, although this methods allows also to address propagation in two dimen-
 280 sions. Moreover, variations of this method have been proposed to overcome the far field assump-
 281 tion, allowing for example to consider the dynamic response close to the excitation point, where
 282 the plane wave assumption does not approximate well the actual vibration field, or to include the
 283 effect of reflections [54, 55].

284 For each investigated frequency, the structural wavenumbers were determined by maximizing
 285 the function:

$$286 \quad \mathcal{F}(\omega, k_x) = \frac{\sum_n s(\omega, x) o^*(\omega, \tilde{k}_x) \Delta x}{\sqrt{\sum_n |s(\omega, x)|^2 \Delta x \sum_n |o(\omega, \tilde{k}_x)|^2 \Delta x}} \quad (9)$$

287 where Δx represents the spacing between two adjacent measurement positions, expressed in
 288 meters, s is the spatial response, and the symbol $*$ represents the complex conjugate.

289 4. Benchmarking of methods

290 In order to evaluate the reliability and the accuracy of the five chosen methods and, more
 291 specifically, of their implementation, a preliminary benchmarking was performed on an ideal

292 numerical data set, not affected by a poor signal to noise ratio, nor by reflections due to dis-
293 continuities of the real structures, nor to experimental errors. To this purpose, FE models were
294 implemented to investigate structural wave propagation in a homogeneous isotropic element, for
295 which both the elastic constants and the wave velocity dispersion relation are well defined. The
296 models were implemented using the *structural mechanics module* of the commercial software
297 Comsol Multiphysics. A time dependent analysis was performed on a 2 m long, 0.5 m wide
298 and 0.012 m thick plasterboard, with density $\rho = 840 \text{ kg/m}^3$, Young modulus $E = 2.80 \text{ GPa}$,
299 Poisson's ratio $\nu = 0.33$, and internal loss factor $\eta = 0.05$. **The structure was modelled as a 3D**
300 **elastic solid. A structured quadrilateral mesh was adopted, with maximum size smaller than 1/6**
301 **of the shorter investigated wavelength.** At one end of the board, a Gaussian pulse, generated at
302 different frequencies, was imposed as prescribed displacement, while a fixed boundary condition
303 was applied to the other end. The dynamic response of the excited board was evaluated on ten
304 pairs of points equally spaced ($\Delta x = 50 \text{ mm}$). The propagation velocity of the flexural wave
305 was thus evaluated by processing each couple of numerical signals by means of the methods
306 described in section 3.1. A frequency-domain analysis (considering a harmonic condition) was
307 also performed on the same model, in order to investigate the structural wavenumbers propa-
308 gating into the plasterboard. A broad-frequency excitation was applied as boundary load to one
309 end of the investigated board and the excitation frequency was swept from 20 Hz to 5000 Hz at
310 steps of 10 Hz. Moreover, in order to investigate the influence of shear deformation and rota-
311 tory inertia, boards with a different thickness were considered: 12 mm, 24 mm, 48 mm, 96 mm.
312 The propagating wavenumbers were determined, according to the approach described in section
313 3.2.2, from the dynamic response evaluated over evenly distributed points with spacing Δx .

314 The results of the wave velocity obtained from the four presented ToF-based approaches,
315 averaged over ten pairs of evaluation points, are compared in Figure 1 with the theoretical dis-
316 persion relation for bending waves. Figure 1d) also reports the wave velocity obtained from the
317 phase difference method, described in section 3.2.1, given that the output of this method is analo-
318 gous to and comparable with the results of ToF-based methods. In fact, even though it can be cat-
319 egorised among the approaches developed within the frequency domain, single frequency pulses
320 are excited into the investigated structure, rather than a broadband signal. These four methods
321 provided reliable results, in perfect agreement with the bending wave velocity computed using
322 Eq. (1). The highest variability, expressed in the graphs in terms of standard deviation by the

323 bar errors, was always found at the lowest investigated frequency, irrespective of the adopted
 324 method. It can be observed that each of these methods can provide accurate results, at least when
 325 applied to numerical and noiseless signals. However, it should be mentioned that each processing
 326 algorithm is sensitive to different parameters which need to be conveniently tweaked, according
 327 to the investigated frequency, in order to properly compare the two signals: for example, the
 328 criteria to identify the same peak of the propagating pulse or the size of time window which has
 329 to be applied in order to avoid influence of reflections. From the average value of the propagation
 330 velocity, the elastic modulus of the plasterboard element was evaluated according to Kirchhoff's
 331 dispersion relation. The results obtained from the different methods are summarised in Table 1
 332 and compared with the target value, represented by the elastic modulus used as input data in the
 333 FE model: $E = 2.80$ GPa.

334 The structural wavenumbers obtained from the frequency-domain FE model for each inves-
 335 tigated element are compared in Figure 2 with the dispersion curves for thin and thick plates,
 336 given in Eq. (1) and (2) respectively. The board's dynamic response was evaluated on a line of
 337 equally spaced points, aligned with the excitation force.

338 As mentioned in Section 3.2.2, the upper limit in the wavenumber domain is dependent upon
 339 the distance Δx between two adjoining evaluation points. This effect is highlighted in Figure 2:
 340 a spacing of $\Delta = 50$ mm, determined the upper wavenumber limit, $k_{max,\Delta=50mm} \approx 31$ rad/m,
 341 which is reached at frequencies around $f = 1000$ Hz in the 12 mm thick beam, $f = 1800$ Hz
 342 in the 24 mm thick beam, $f = 3000$ Hz in the 48 mm thick beam, and $f = 4200$ Hz in the
 343 96 mm thick beam. On the other hand, reducing the distance between the evaluation points
 344 to $\Delta x = 10$ mm allowed to accurately approximate the propagating wavenumber in the entire
 345 frequency domain, since $k_{max,\Delta=10mm} \approx 157$ rad/m. The wavenumber resolution δk is associated
 346 to the evaluation length L_e , i.e. the span covered by the array of measurement points: $\delta k = 2\pi/L_e$.
 347 The first evaluation point was chosen at 50 mm from the point load while the last position was
 348 evaluated at 50 mm from the opposite end, resulting in an evaluation length $L_e = 1.8$ m, which
 349 determined a wavenumber resolution: $\delta k = 3.5$ rad/m. As shown in Figure 2, as the thickness of
 350 the investigated structure increases, the dispersion relations for thin and thick plates increasingly
 351 deviate, since shear deformation and rotatory inertia have a greater influence. The elastic moduli
 352 of the investigated structures, obtained from the wave correlation method, are listed in Table 1.
 353 It is clear that, as the thickness of the element increases, the thin plate dispersion relation fails to

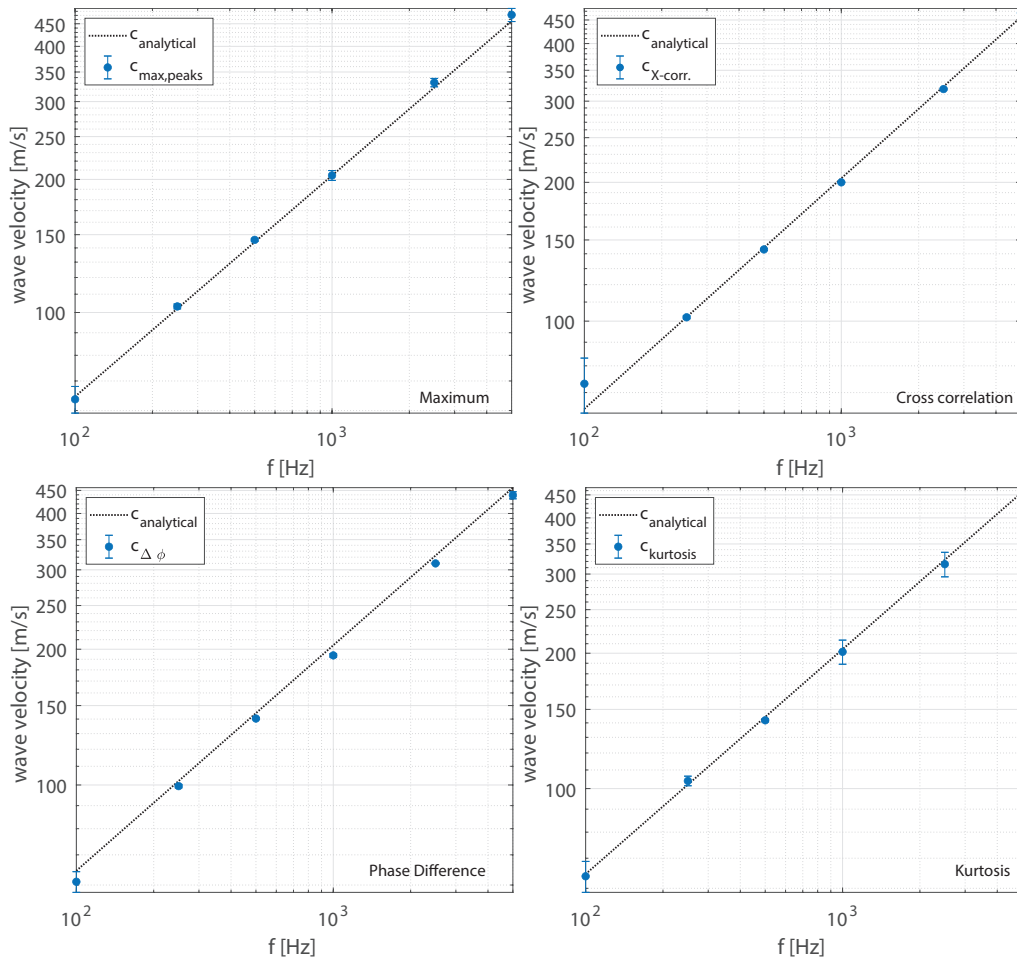


Figure 1: Benchmark results: comparison between the analytical dispersion curve for bending waves and the wave propagation velocity evaluated from time domain FE simulations.

354 evaluate the elastic modulus since the influence of shear deformation and rotatory inertia cannot
 355 be neglected. On the other hand, Mindlin's dispersion relation is suitable to accurately determine
 356 the elastic properties both for thin and thick elements. Moreover, the shear modulus can also
 357 be accurately evaluated if the investigation is performed up to high frequencies, where it has a
 358 significant influence on the structure's dynamic response.

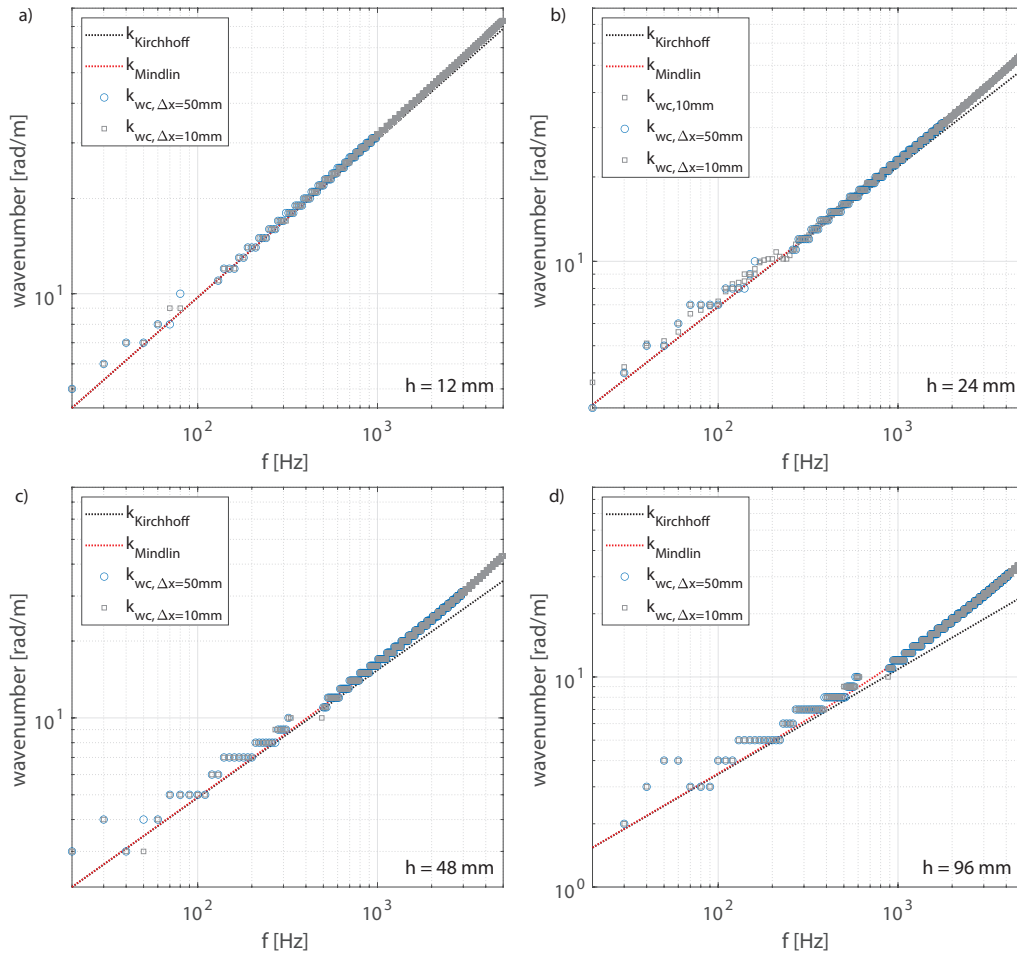


Figure 2: Benchmark results: comparison between the analytical wavenumber dispersion curves for thin and thick plates and the results obtained from a wave correlation approach applied to: a) 12 mm thick plasterboard; b) 24 mm thick plasterboard; c) 48 mm thick plasterboard; d) 96 mm thick plasterboard.

359 5. Experimental analysis

360 The measurements were performed in the Acoustic Laboratory of the Department of Indus-
 361 trial Engineering at the University of Bologna. A CLT floor was installed in the sound transmis-
 362 sion test facilities with suppressed flanking transmission. The overall dimension of the floor is
 363 4.44 x 2.82 m; it is made of four pieces of 5-ply 160 mm CLT elements (40/20/40/20/40), con-
 364 nected by fully threaded screws fixed with an angle of 45 degrees and sealed at the junctions. The

Table 1: Benchmark results: elastic properties of plasterboard element.

Method	E [GPa]	ϵ_E [%]	G [GPa]	ϵ_G [%]	FEM values		
					E [GPa]	G [GPa]	
ToF	Max Peak	2.93	4.8				
	X-corr.	3.07	9.6				
	$\Delta\phi$	2.39	14.7				
	Kurtosis	2.62	6.3				
Wave Correlation	Kirch. $h_{12\text{mm}}$	2.30	17.9			2.80	
	Mindl. $h_{12\text{mm}}$	2.57	8.2	1.65	52.8		1.08
	Kirch. $h_{24\text{mm}}$	1.92	31.4				
	Mindl. $h_{24\text{mm}}$	2.56	8.7	1.14	5.6		
	Kirch. $h_{48\text{mm}}$	1.45	48.1				
	Mindl. $h_{48\text{mm}}$	2.57	8.6	1.07	0.9		
	Kirch. $h_{96\text{mm}}$	0.98	67.5				
	Mindl. $h_{96\text{mm}}$	2.61	6.9	1.09	0.9		

365 floor is simply supported on the two longer edges and has free edges in the shorter dimension.
366 The measurements that are presented in the paper refer to the principal direction parallel to grain,
367 characterised by the higher bending stiffness. The density of the panel is $\rho_{mean} = 420 \text{ kg/m}^3$; the
368 Young's moduli parallel to grain and perpendicular to grain of each timber beam composing
369 the layers are respectively $E_{0,mean} = 12 \text{ GPa}$ and $E_{90,mean} = 0.37 \text{ GPa}$, while the shear modulus is
370 $G_{mean} = 0.69 \text{ GPa}$, as declared in the technical datasheet of the CLT panel.

371 The measurement setups used for the time domain methods and the frequency domain meth-
372 ods are different. A detailed description of the two procedures is provided below.

373 5.1. Time domain measurements

374 For the measurements in the time domain, the measurement equipment consisted of an elec-
375 trodynamic shaker LDS V201 with an LDS PA25E power amplifier, an RME Fireface 802
376 soundcard, 1/2" ICP accelerometers (PCB Piezotronic) powered by a signal conditioner PCB
377 Piezotronics 482C. For each investigated frequency, Gaussian-modulated sinusoidal pulses, gen-



Figure 3: Experimental setup used to investigate the CLT floor installed in the laboratory facility. a) side view: the shaker is mounted on a tripod with rubber supports on the three feet; b) top view: three accelerometers are mounted along a wave propagation direction in line with the excitation point.

378 erated in Matlab[®] for each investigated frequency, were run and recorded through REAPER[®]
379 software, with a sampling frequency of 192 kHz. Two accelerometers were fixed to the floor us-
380 ing wax, at a mutual distance of 0.40 m along the strands of the face ply, as pictured in Figure 4.
381 The pulses were centered at frequencies that ranged from 100 to 1500 Hz, with a step of 100 Hz.
382 Each pulse was repeated 5 times. Measurements were done using three accelerometers at a time,
383 and acquiring the input signal from an impedance head which was mounted on the shaker; this
384 allowed to time-align all the measurements.

385 5.2. Frequency domain measurements

386 Measurements in the frequency domain were performed using the same electrodynamic
387 shaker and accelerometers presented above. The measurements were controlled by means of

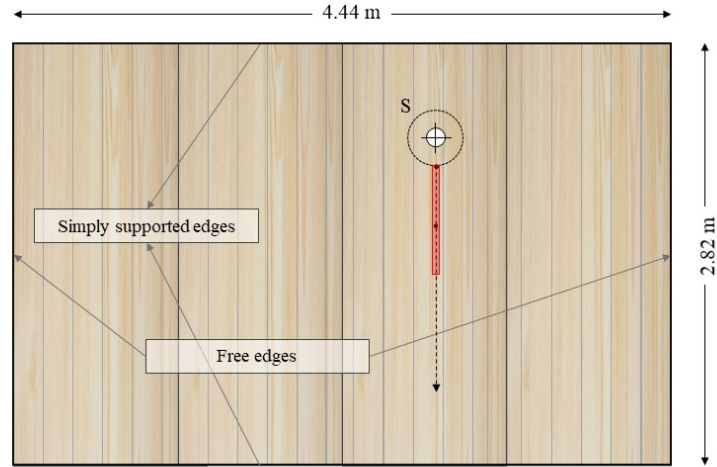


Figure 4: Scheme of the layout of the floor and the position of the accelerometers.

388 an in-house implemented LabVIEW software and the NI USB-4431 DAQ (data acquisition).
 389 Measurement positions spanned a total length of 0.72 m, the spacing between the accelerometers
 390 being 0.02 m. Also in this case, accelerometers were fixed to the floor using wax. The panel
 391 was excited driving the shaker with an exponential sine sweep. Each acquired signal was then
 392 convolved with the inverse filter of the sine sweep in order to obtain its impulse responses (IR),
 393 using a consolidated experimental technique [56]. The complex plate response spectra $s(\omega, x_i)$,
 394 measured over each point i , were obtained from the IR by means of a FFT algorithm. The acqui-
 395 sition of the signals was extremely fast, though limited by the number of accelerometers available
 396 which, with the given equipment, determined the maximum sampling frequency available for
 397 each channel.

398 6. Results and discussion

399 All the measurements were post-processed in Matlab[®]. Each signal was filtered using a
 400 low-pass filter to cancel high-frequency noise. Each method has specific settings that must be
 401 tweaked in order to bring the method to convergence.

402 In the analysis proposed, the maximum peak method is implemented by identifying the first
 403 relative maximum, to reduce the possibility of interference phenomena due to reflections. For
 404 each pulse, the prominence of the peaks was used as an input parameter to correctly detect the

Table 2: Estimate of the elastic modulus E and the shear modulus G from the non-linear fit performed using Mindlin's models.

Method	$E_{Mindlin}$ [GPa]	$G_{Mindlin}$ [GPa]
Maximum	3.37	0.300
Cross-correlation	2.48	0.280
Kurtosis	5.46	0.212
Phase difference	0.307	n. a.
Wave correlation	3.62	0.456

405 first peak. The results obtained from the kurtosis and the phase difference methods were strongly
 406 influenced by the width of the window and the length of the signal respectively.

407 The dispersion relations of the CLT plate obtained with the first four methods are reported
 408 in Figure 5. Using the first set of measurements (time domain pulses), a total of 5 measures
 409 were available for each frequency, as explained above. The results were then averaged and the
 410 standard deviation was calculated. These data were finally used to perform a non linear fit to
 411 Mindlin's model, based on the first order shear deformation plate theory. Although it can be
 412 argued that this theory neglects several high-order propagation modes, yet it is still one of the
 413 most used approaches implemented for vibro-acoustic prediction models, since it allows for a
 414 good approximation of the transverse displacement, which is the one of greatest importance to
 415 evaluate the fluid-structure interaction.

416 The raw data obtained by the application of the methods show that these measurements are
 417 affected by multiple reflections that are generated within the element. For most of the methods
 418 analysed, the dispersion of the data is relevant. The methods that provide lower dispersion of the
 419 data, frequency-by-frequency, are the maximum peak method and the cross-correlation method.

420 The results show that the method based upon the identification of the maximum peak provide
 421 the most consistent results in the whole frequency range of interest, with a limited dispersion of
 422 the data. A low variation was also found with the cross-correlation method. However, a good
 423 agreement between the fitted curve and the experimental data could be found only above 600
 424 Hz, while at the lower frequencies high fluctuations were observed. The results obtained from

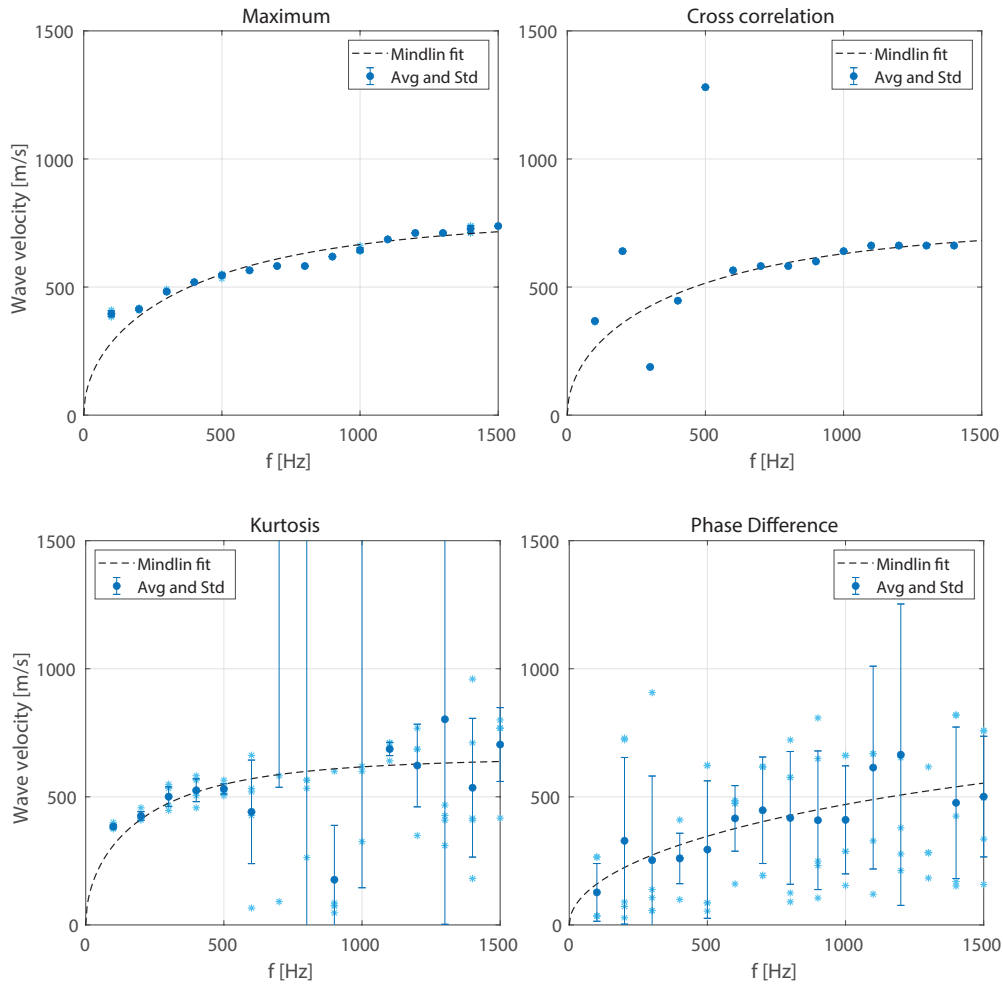


Figure 5: Measurement of bending waves velocity obtained with the four different methods based upon the acquisition of the pulses. Grey starred markers represent single measurements, while blue markers and bars represent the mean value and the standard deviation.

425 the kurtosis and the phase difference methods were characterised by a high deviation. While the
 426 kurtosis approach allowed to identify wave velocities consistent with the values obtained from
 427 the maximum-peak and the cross-correlation methods up to 500 Hz, the results determined with
 428 the phase difference method were characterised by high uncertainty within the entire frequency
 429 range.

430 The real part of the wavenumber determined with the wave correlation method is displayed
 431 in Figure 6. The graph on the left-hand side represents the real part of the function \mathcal{F} , given

432 in Eq. 9, evaluated for a given value of the imaginary component of the wavenumber in the
 433 frequency range 0-3000 Hz. The maxima of the function are plotted using markers and were
 434 superimposed to the image. These points were then fitted using Mindlin's model, represented
 435 with a dashed red line. For a better readability, the experimental wavenumbers and the fitted
 436 curve are also reported in the right-hand side graph, with more suitable limits for the y-axis. The
 437 experimental wavenumber exhibited significant fluctuations in the lower frequency range, which
 438 is related to the evaluation length L_e over which the structure's dynamic response was measured.
 439 Moreover, in the range approximately between 1500 Hz and 2000 Hz the experimental wavenum-
 440 bers seem to asymptotically flatten out. This behaviour, related to lateral reflections, could be
 441 misleading. In fact, higher wavenumbers were measured at higher frequencies. The possibility
 442 to investigate a broad frequency range allowed to perform a robust fitting with an analytic dis-
 443 persion relation, even though experimental data exhibited fluctuations and high uncertainty in
 444 particular ranges of frequency.

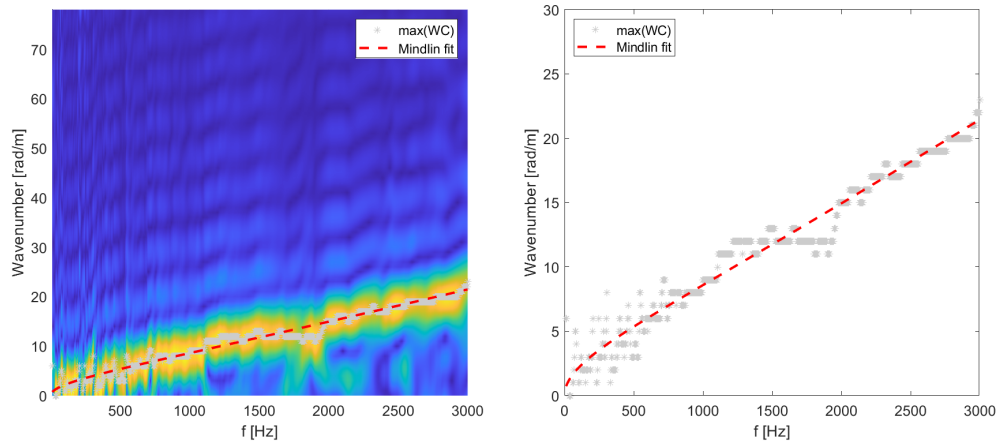


Figure 6: Dispersion image calculated with the wave correlation method: identification of the maxima to fit the dispersion relation (left) and estimate of the wave velocity (right).

445 For the five implemented methods, **effective elastic and shear properties** E and G were con-
 446 sidered the optimisation variables of the non-linear fitting process, whereas Poisson's ratio was
 447 assumed to be a constant value of $\nu = 0.3$. The optimised variables obtained with each methods
 448 are summarised in Table 2. Moreover, in order to investigate the influence of the strong assump-
 449 tion made on the value of the Poisson ratio, a sensitivity analysis was conducted to estimate the

450 effect that the hypothesised value of ν has on these quantities. To this aim, the wave correlation
 451 method was assumed as a reference and the experimental data were fitted with Mindlin's dis-
 452 persion relation changing the value of the Poisson's ratio ν . The corresponding **effective elastic**
 453 **properties E and G** are then plotted on a two y-axes plot in Figure 7. **Correlation between Pois-**
 454 **son's ratio and E and G** highlighted a non-linear relationship of these effective elastic properties
 455 **over ν** . By increasing the Poisson's ratio from $\nu = 0.1$ up to $\nu = 0.45$, a larger variation was
 456 found on $E(\nu = 0.45) \approx 0.8E(\nu = 0.1)$ rather than on $G(\nu = 0.45) \approx 0.9G(\nu = 0.1)$.

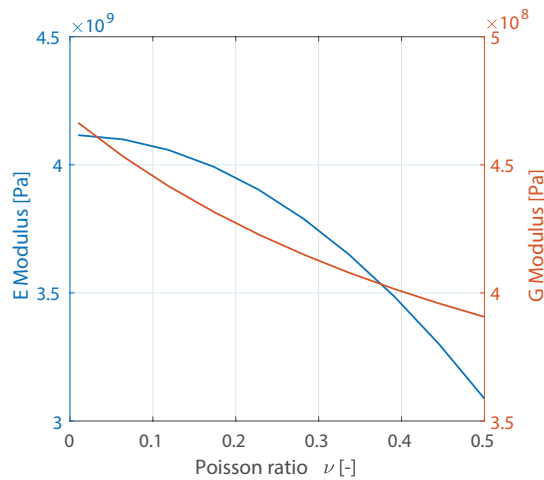


Figure 7: Sensitivity analysis of Poisson's ratio ν on the estimate of the elastic modulus E and on the shear modulus G .

457 The most intuitive approach to compare results obtained from methods for the characteri-
 458 sation of the elastic properties would be to compare the results with a target value, as done in
 459 Section 4. The data sheet of CLT elements provides the elastic properties of the lamellas which
 460 constitute each ply of the panel. Different methods can be found in the literature to estimate the
 461 equivalent Young modulus of a CLT element from the elastic properties of the timber lamellas
 462 and the number of plies of the panel [15]. Annex B of Eurocode 5 [57], for example, provides a
 463 method, based on the mechanically jointed beam theory, also known as Gamma Method. The val-
 464 ues it provide can be used to perform structural analyses in static (or quasi-static) conditions. On
 465 the other hand, Mindlin's theory for thick plates was considered in this study, since it is widely
 466 used in vibro-acoustic analysis. **It should be stressed that E and G are not the elastic constants**
 467 **of the CLT panel; rather, they represent apparent elastic properties to approximate the dynamic**
 468 **response of such a structure described as an equivalent homogeneous thick plate.** The equivalent

469 properties are suitable input data in homogenisation approaches as long as the assumptions made
470 to derive them and the ones of the method of analysis are the same. For this reason, it is not
471 possible to directly compare the elastic **constants** of the CLT panel derived by using the afore-
472 mentioned methods, with the **effective elastic properties** determined in this study. Therefore, the
473 investigated methods were compared in the following in terms of the wave velocity dispersion
474 curves.

475 The comparison between methods based upon numerical input data showed an extremely
476 good agreement under ideal conditions, the only limitations arising from physical problems (e.g.
477 the Nyquist limit or transition from thin plate to thick plate behavior). Conversely, experimental
478 measurements on a non-homogeneous plate are affected by a significantly larger variability. In
479 Figure 8, the different methods are compared in terms of wave velocity determined for each in-
480 vestigated frequency. In particular, the graph on the left-hand side shows the experimental wave
481 velocities directly determined from each method, or computed from the experimental wavenum-
482 bers. It is clear how all the methods are strongly affected by the complex structure on the CLT
483 panel, generating multiple reflections in all the directions. However, some of them allow the
484 identification of a certain trend while others totally fail to provide useful results to characterise
485 wave propagation. The graph given on the right-hand side of Figure 8 shows the fitted dispersion
486 curves calculated from the experimental data. The results obtained with the wave correlation are
487 considered the most reliable, since they were fitted over a wider frequency range, up to 3000 Hz.
488 The curve fitting of the experimental data obtained from the other methods, which only allowed
489 the experimental investigation of a limited frequency range, was strongly influenced by the im-
490 possibility to correctly evaluate the effect of the shear deformations, which are significant above
491 the shear wave crossover frequency, as defined in [37]. On the other hand, the frequency range
492 of applicability of these methods could be extended by reducing the distance between the pair of
493 transducers. Methods based on phase information are much more sensitive to the sensors spac-
494 ing: several spacings should be considered to span the whole frequency range of interest. This
495 consideration can be also a criterion to evaluate the efficiency of the use of one method compared
496 to others. The wave correlation results provided clear information about wave propagation at the
497 highest frequencies, and a trend which is in good agreement with the velocities obtained for the
498 maximum-peak and the cross correlation methods up to 1500 Hz. In the graph on the right side
499 of Figure 8, the curve labelled as *Maximum-WC* was obtained by fitting with Mindlin's disper-

500 sion relation a combined data set which considers the maximum-peak results up to 1500 and the
 501 wave correlation results from 1500 Hz up to 3000 Hz. This curve is in perfect agreement with
 502 the one obtained considering only the wave correlation based results, proving the reliability of
 503 this methods and the robustness of the curve fitting on the experimental data evaluated over an
 504 appropriate frequency range.

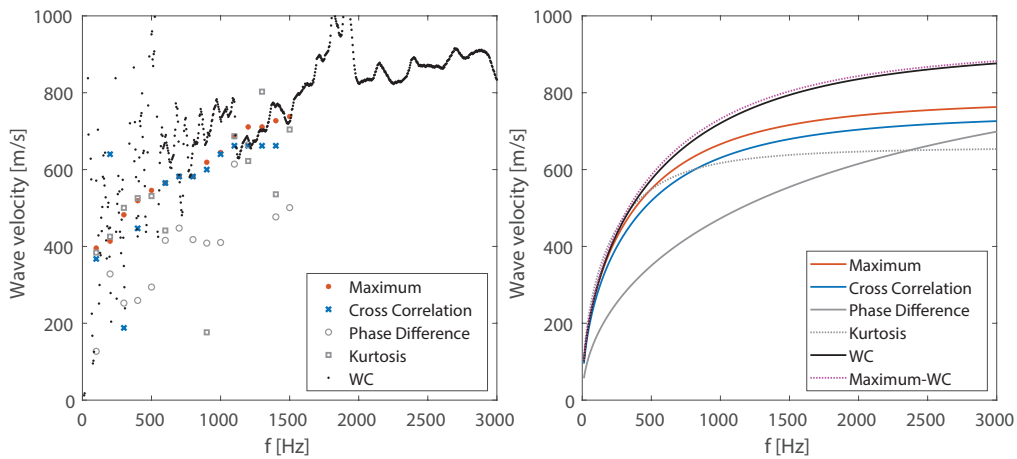


Figure 8: Wave velocity dispersion curves: experimental results determined with the five methods considered (left-hand side); curve fitting of the experimental data with Mindlin's dispersion relation (right-hand side).

505 It is finally worth comparing the performance of the methods considering the time required
 506 for the installation of each setup and for the gathering of the measures. The setup labelled in
 507 Section 5 as time-domain used three accelerometers at a time; a total of 16 frequencies were
 508 scanned, even though undersampling is possible, as discussed above. Each pulse was generated
 509 five times, for a total duration of 5 seconds each measurement (time interval of 1 sec between
 510 the pulses); since three measurement positions were considered for averaging, that leads to the
 511 processing of 80 signals. Conversely, frequency-domain measurements required a larger number
 512 of measurement positions, which can be performed also non-simultaneously, guaranteeing
 513 the same relative phase relationship with a reference transducer. The array used in this study
 514 consisted of 36 measurement points, performed using 3 accelerometers simultaneously. A 10
 515 seconds sine sweep was averaged 5 times. The amount of time required to evaluate the structural
 516 dynamic response over an adequate length with a suitable spacing, is therefore dependent upon
 517 the availability of transducers and acquisition channels. The computational time required to run

518 the algorithm is negligible compared to time required to perform the measurement.

519 **Conclusions**

520 The availability of an experimental estimate of the bending wavenumber is of great inter-
521 est for vibro-acoustic modeling. Among the methods available in the literature, it is relevant to
522 understand which methods are better suited to study different elements characterised by a pecu-
523 liar dynamic response. The present study benchmarks methods for the extraction of dispersion
524 relations on CLT elements, non homogeneous and orthotropic plates, focusing on the real com-
525 ponent of the propagating wavenumber, **from which apparent effective elastic properties could**
526 **be evaluated, by means of a non-linear data fitting based on Mindlin's theory for thick plates.**
527 Five methods were selected, which are characterised by different experimental setups. They
528 were first tested on an FE-simulated test signal; the results returned a good match between all
529 results. Then, the methods were applied to signals measured on a CLT plate; here, the limita-
530 tions characterizing each method emerged in relation to the sensitivity to background noise, to
531 multiple reflections and to the ability to automatise the identification of optimal input parame-
532 ters for a proper estimate. Among all methods tested, the maximum peak, implemented on the
533 first relative maximum, the cross-correlation and the wave correlation method provided the most
534 accurate results. Nevertheless, considering the time required for the measurement of the single
535 pulses, the wave correlation method could be the most satisfactory method, given the availability
536 of an array of transducers. This method, allowing to investigate a wider frequency range with
537 a smaller computational effort, is useful for a proper estimate of **an effective shear modulus G ,**
538 **which is strongly affected by the propagating wavenumber at mid-high frequencies.** Reproducibil-
539 ity studies on CLT panels with different thickness and layer layup will be addressed in the future
540 to verify the consistency of these results.

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Declaration of Interest Statement

Manuscript title: “Determination of the dispersion relation in Cross-Laminated Timber plates: benchmarking of time- and frequency-domain methods.”

Authors: Federica Morandi, Andrea Santoni, Patrizio Fausti, Massimo Garai

On behalf of the authors of this manuscript, I declare that the authors have no known competing financial interests or personal relationships that could have appeared to influence the work reported in this paper.

Sincerely,

Dr. Federica Morandi



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Author Agreement Statement

Manuscript title: “Determination of the dispersion relation in Cross-Laminated Timber plates: benchmarking of time- and frequency-domain methods.”

Authors: Federica Morandi, Andrea Santoni, Patrizio Fausti, Massimo Garai

On behalf of the authors of this manuscript, I declare that this manuscript is original, has not been published before and is not currently being considered for publication elsewhere.

I confirm that the manuscript has been read and approved by all named authors and that there are no other persons who satisfied the criteria for authorship but are not listed. We further confirm that the order of authors listed in the manuscript has been approved by all of us. We understand that the Corresponding Author is the sole contact for the Editorial process. She is responsible for communicating with the other authors about progress, submissions of revisions and final approval of proofs

Sincerely,

Dr. Federica Morandi